
FHWA Bridge Inspector's Manual: *Bridge Inspection Reporting*

Course No: S05-015

Credit: 5 PDH

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Chapter 4

Bridge Inspection Reporting

Topic 4.1 Structure Inventory

4.1.1

Introduction

A good bridge inspection reporting system is essential to document bridge conditions and to protect the public's safety and investment in bridge structures. It is, therefore, essential that bridge inspection data be clear, accurate, and complete, since it is an integral part of the lifelong record file of the bridge.

Because of the requirements that are fulfilled in accordance with the National Bridge Inspection Standards (NBIS), it is necessary to employ a uniform bridge inspection reporting system. A uniform reporting system is essential to evaluate the condition of a structure correctly and efficiently. It is a valuable aid in establishing maintenance priorities and replacement priorities, and in determining structure capacity and the cost of maintaining the nation's bridges. Consequently, importance of the reporting system cannot be overemphasized. Success of any bridge inspection program is dependent upon its reporting system.

4.1.2

FHWA Structure Inventory, Appraisal and Condition Ratings

The FHWA *Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges (FHWA Coding Guide)* is used for defining the bridge inventory and the items to be used to collect information on the overall condition of the deck, superstructure, substructure, and channel. The data is reported to FHWA in accordance with the *FHWA Coding Guide*. It is not an inspection guide. Each state may use its own coding scheme, provided that the data is directly translatable into the format of the *FHWA Coding Guide*. In other words, the states are responsible for having the capability to obtain, store, and report certain information about bridges, for collection by FHWA as requested.

The Structure Inventory and Appraisal (SI&A) sheet is a tabulation of information that is submitted for each individual structure (see Figure 4.1.1).

For the small structures and culverts that are less than or equal to 20 feet, some states still collect the inventory information and generate a "local" database.

It is important to note that the SI&A sheet is not an inspection form. Rather, it is a summary sheet of bridge data required by the FHWA to effectively monitor and manage the National Bridge Inspection Program and the Highway Bridge Program.

Substitutes for the SI&A Sheet

There are suitable substitutes for the SI&A sheet. Some states simply reprint the federal form with the same items and item numbers. A few states have elaborate Bridge Management Systems (BMS) with different item numbers that collect all

the data listed on the SI&A form plus additional items not reported to the FHWA (see Figures 4.1.1 through 4.1.5).

Data Entry Requirements For routine, in-depth, fracture critical member, underwater, damage and special inspections, the NBIS requires entry of the SI&A data into the State or Federal agency inventory within 90 days of the date of inspection for State or Federal agency bridges and within 180 days of the date of inspection for all other bridges.

For existing bridge modifications that alter previously recorded data and for new bridges, the NBIS requires entry of the SI&A data into the State or Federal agency inventory within 90 days after the completion of the work for State or Federal agency bridges and within 180 days after the completion of the work for all other bridges.

For changes in load restriction or closure status, the NBIS requires entry of the SI&A data into the State or Federal agency inventory within 90 days after the change in status of the structure for State or Federal agency bridges and within 180 days after the change in status of the structure for all other bridges.

FHWA

Office of Asset Management
DOT

Structure Inventory and Appraisal Sheet

Bridge Key: 11 0013		Agency ID: 11 0013		Sufficiency Rating: 96.8																																																																																																										
IDENTIFICATION State 1: 06 California Struc Num 8: 11 0013 Facility Carried 7: STATE ROUTE 162 Location 9: 03-GLE-162--73.55 Rte.(On/Under)5A: Route On Structure Rte. Signing Prefix 5B: 3 State Hwy Level of Service 5C: 1 Mainline Rte. Number 5D: 00162 Directional Suffix 5E: 0 N/A (NBI) % Responsibility : Unknown SHD District 2: District 3 County Code 3: (11)GLENN Place Code 4: Unknown Kilometer Post 11: 73.6 km Feature Intersected 6: BRUSH CANAL Latitude 16: 39d 31' 18" Longitude 17: 122d 03' 42" Border Bridge Code 98: Unknown (P) Border Bridge Number 99: Unknown			INSPECTION Frequency 91: 24 months Inspection Date 90: 10/28/1997 Next Inspection: 10/28/1999 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA SI Frequency 92C: NA SI Date 93C: NA Next SI: NA Element Frequency: 24 months Element Inspection Date: 12/11/1997 Next Elem. Insp. Due: 10/28/1999																																																																																																											
STRUCTURE TYPE AND MATERIALS Number of Approach Spans 46: 0 Number of Spans Main Unit 45: 2 Main Span Material/Design 43A/B: 2 Concrete Continuous 01 Slab Deck Type 107: 1 Concrete-Cast-in-Place Wearing Surface 108A: 1 Monolithic Concrete Membrane 108B: 0 None Deck Protection 108C: None			CLASSIFICATION Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: No bridge exists Direction of Traffic 102: 2 2-way traffic Temporary Structure 103: Unknown (NBI) Highway System 104: 0 Not on NHS NBIS Length 112: Long Enough Toll Facility 20: 3 On free road Functional Class 26: 06 Rural Minor Arterial Historical Significance 37: 5 Not eligible for NRHP Owner 22: 1 State Highway Agency Custodian 21: 1 State Highway Agency																																																																																																											
AGE AND SERVICE Year Built 27: 1963 Year Reconstructed 106: Unknown Type of Service on 42A: 1 Highway Type of Service under 42B: 5 Waterway Lanes on 28A: 2 Lanes Under 28B: 0 Detour Length 19: 13 km ADT 29: 1,900 Truck ADT 109: 12 % Year of ADT 30: 1994			CONDITION Deck 58: 7 Good Super 59: 7 Good Sub 60: 7 Good Culvert 62: N N/A (NBI) Channel/Channel Protection 61: 8 Protected																																																																																																											
GEOMETRIC DATA Length Max Span 48: 6.40 m Structure Length 49: 13.70 m Curb/Sdwk Width L 50A: 0.00 m Curb/Sidewalk Width R 50B: 0.00 m Width Curb to Curb 51: 10.80 m Width Out to Out 52: 11.30 m Approach Roadway Width 32: 9.80 m Median 33: 0 No median (w/ shoulders) Deck Area: 155.00 m ² Skew 34: 5.00 ° Structure Flared 35: 0 No flare Minimum Vertical Clearance Over Bridge 53: 99.99 m Minimum Vertical Underclearance Reference 54A: N Feature not hwy or RR Minimum Vertical Underclearance 54B: 00.00 m Minimum Lateral Underclearance Reference R 55A: N Feature not hwy or RR Minimum Lateral Underclearance R 55: 99.90 m Minimum Lateral Underclearance L 56: 00.00 m			LOAD RATING AND POSTING Inventory Rating Method 65: 1 LF Load Factor Operating Rating Method 63:1 LF Load Factor Inventory Rating 66: MS20.7 Operating Rating 64: MS34.2 Design Load 31: 5 MS 18 (HS 20) Posting 70: 5 At/Above Legal Loads Posting status 41: A Open, no restriction																																																																																																											
ELEMENT CONDITION STATE DATA <table border="1" style="width:100%; border-collapse: collapse; font-size: small;"> <thead> <tr> <th>Str Unit</th> <th>Elm/Env</th> <th>Description</th> <th>Units</th> <th>Total Qty</th> <th>% in 1</th> <th>Qty. St. 1</th> <th>% in 2</th> <th>Qty. St. 2</th> <th>% in 3</th> <th>Qty. St. 3</th> <th>% in 4</th> <th>Qty. St. 4</th> <th>% in 5</th> <th>Qty. St. 5</th> </tr> </thead> <tbody> <tr> <td>2</td> <td>38/2</td> <td>Bare Concrete Slab</td> <td>sq.m.</td> <td>160</td> <td>100 %</td> <td>160</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> </tr> <tr> <td>2</td> <td>205/2</td> <td>R/Conc Column</td> <td>ea.</td> <td>5</td> <td>100 %</td> <td>5</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> </tr> <tr> <td>2</td> <td>215/2</td> <td>R/Conc Abutment</td> <td>m.</td> <td>23</td> <td>100 %</td> <td>23</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> </tr> <tr> <td>2</td> <td>226/2</td> <td>P/S Conc Submgd Pile</td> <td>ea.</td> <td>13</td> <td>100 %</td> <td>13</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> </tr> <tr> <td>2</td> <td>333/2</td> <td>Other Bridge Railing</td> <td>m.</td> <td>35</td> <td>100 %</td> <td>35</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> </tr> <tr> <td>2</td> <td>358/2</td> <td>Deck Cracking SmFlag</td> <td>ea.</td> <td>1</td> <td>100 %</td> <td>1</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> <td>0 %</td> <td>0</td> </tr> </tbody> </table>			Str Unit	Elm/Env	Description	Units	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5	2	38/2	Bare Concrete Slab	sq.m.	160	100 %	160	0 %	0	0 %	0	0 %	0	0 %	0	2	205/2	R/Conc Column	ea.	5	100 %	5	0 %	0	0 %	0	0 %	0	0 %	0	2	215/2	R/Conc Abutment	m.	23	100 %	23	0 %	0	0 %	0	0 %	0	0 %	0	2	226/2	P/S Conc Submgd Pile	ea.	13	100 %	13	0 %	0	0 %	0	0 %	0	0 %	0	2	333/2	Other Bridge Railing	m.	35	100 %	35	0 %	0	0 %	0	0 %	0	0 %	0	2	358/2	Deck Cracking SmFlag	ea.	1	100 %	1	0 %	0	0 %	0	0 %	0	0 %	0	APPRAISAL Bridge Rail 36A: 1 Meets Standards Approach Rail 36C: 0 Substandard Transition 36B: 0 Substandard Approach Rail Ends 36D: 0 Substandard Str. Evaluation 67: 7 Above Min Criteria Deck Geometry 68: 6 Equal Min Criteria Underclearance, Vertical and Horizontal 69: N Not applicable (NBI) Waterway Adequacy 71: 8 Equal Desirable Approach Alignment 72: 8 Equal Desirable Crit Scour Critical 113: 6 Calcs not made		
Str Unit	Elm/Env	Description	Units	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5																																																																																																
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2	358/2	Deck Cracking SmFlag	ea.	1	100 %	1	0 %	0	0 %	0	0 %	0	0 %	0																																																																																																
PROPOSED IMPROVEMENTS Bridge Cost 94: \$ 0 Type of Work 75: Unknown (P) Roadway Cost 95: \$ 0 Length of Improvement 76: 00.00 m Total Cost 96: \$ 0 Future ADT 114: 2,900 Year of Cost Estimate 97: Unknown Year of Future ADT 115: 2010			NAVIGATION DATA Navigation Control 38: 0 Permit Not Required Vertical Clearance 39: 0.00 m Horizontal Clearance 40: 0.00 m Pier Protection 111: Unknown (NBI) L/R Bridge Vertical Clearance 116:																																																																																																											
INSP001_Inspection_SIA_Metric																																																																																																														

Figure 4.1.1 Example SI&A Sheet with Element Level Data

Appendix A

OMB No. 2125-0501

Structure Inventory and Appraisal Sheet

NATIONAL BRIDGE INVENTORY - - - - - STRUCTURE INVENTORY AND APPRAISAL 10/15/94

***** IDENTIFICATION *****

(1) STATE NAME - _____ CODE _____
 (8) STRUCTURE NUMBER _____ # _____
 (5) INVENTORY ROUTE (ON/UNDER) - _____ = _____
 (2) HIGHWAY AGENCY DISTRICT _____
 (3) COUNTY CODE _____ (4) PLACE CODE _____
 (6) FEATURES INTERSECTED - _____
 (7) FACILITY CARRIED - _____
 (9) LOCATION - _____
 (11) MILEPOINT/KILOMETERPOINT _____
 (12) BASE HIGHWAY NETWORK - _____ CODE _____
 (13) LRS INVENTORY ROUTE & SUBROUTE # _____
 (16) LATITUDE _____ DEG _____ MIN _____ SEC
 (17) LONGITUDE _____ DEG _____ MIN _____ SEC
 (98) BORDER BRIDGE STATE CODE _____ % SHARE _____ %
 (99) BORDER BRIDGE STRUCTURE NO. # _____

***** STRUCTURE TYPE AND MATERIAL *****

(43) STRUCTURE TYPE MAIN: MATERIAL - _____
 TYPE - _____ CODE _____
 (44) STRUCTURE TYPE APPR: MATERIAL - _____
 TYPE - _____ CODE _____
 (45) NUMBER OF SPANS IN MAIN UNIT _____
 (46) NUMBER OF APPROACH SPANS _____
 (107) DECK STRUCTURE TYPE - _____ CODE _____
 (108) WEARING SURFACE / PROTECTIVE SYSTEM:
 A) TYPE OF WEARING SURFACE - _____ CODE _____
 B) TYPE OF MEMBRANE - _____ CODE _____
 C) TYPE OF DECK PROTECTION - _____ CODE _____

***** AGE AND SERVICE *****

(27) YEAR BUILT _____
 (106) YEAR RECONSTRUCTED _____
 (42) TYPE OF SERVICE: ON - _____
 UNDER - _____ CODE _____
 (28) LANES: ON STRUCTURE _____ UNDER STRUCTURE _____
 (29) AVERAGE DAILY TRAFFIC _____
 (30) YEAR OF ADT _____ (109) TRUCK ADT _____ %
 (19) BYPASS, DETOUR LENGTH _____ KM

***** GEOMETRIC DATA *****

(48) LENGTH OF MAXIMUM SPAN _____ M
 (49) STRUCTURE LENGTH _____ M
 (50) CURB OR SIDEWALK: LEFT _____ M RIGHT _____ M
 (51) BRIDGE ROADWAY WIDTH CURB TO CURB _____ M
 (52) DECK WIDTH OUT TO OUT _____ M
 (32) APPROACH ROADWAY WIDTH (W/SHOULDERS) _____ M
 (33) BRIDGE MEDIAN - _____ CODE _____
 (34) SKEW _____ DEG (35) STRUCTURE FLARED _____
 (10) INVENTORY ROUTE MIN VERT CLEAR _____ M
 (47) INVENTORY ROUTE TOTAL HORIZ CLEAR _____ M
 (53) MIN VERT CLEAR OVER BRIDGE RDWY _____ M
 (54) MIN VERT UNDERCLEAR REF - _____ M
 (55) MIN LAT UNDERCLEAR RT REF - _____ M
 (56) MIN LAT UNDERCLEAR LT REF - _____ M

***** NAVIGATION DATA *****

(38) NAVIGATION CONTROL - _____ CODE _____
 (111) PIER PROTECTION - _____ CODE _____
 (39) NAVIGATION VERTICAL CLEARANCE _____ M
 (116) VERT-LIFT BRIDGE NAV MIN VERT CLEAR _____ M
 (40) NAVIGATION HORIZONTAL CLEARANCE _____ M

***** CLASSIFICATION *****

(112) NBIS BRIDGE LENGTH - _____
 (104) HIGHWAY SYSTEM - _____
 (26) FUNCTIONAL CLASS - _____
 (100) DEFENSE HIGHWAY - _____
 (101) PARALLEL STRUCTURE - _____
 (102) DIRECTION OF TRAFFIC - _____
 (103) TEMPORARY STRUCTURE - _____
 (105) FEDERAL LANDS HIGHWAYS - _____
 (110) DESIGNATED NATIONAL NETWORK - _____
 (20) TOLL - _____
 (21) MAINTAIN - _____
 (22) OWNER - _____
 (37) HISTORICAL SIGNIFICANCE - _____

***** CONDITION *****

(58) DECK _____
 (59) SUPERSTRUCTURE _____
 (60) SUBSTRUCTURE _____
 (61) CHANNEL & CHANNEL PROTECTION _____
 (62) CULVERTS _____

***** LOAD RATING AND POSTING *****

(31) DESIGN LOAD - _____ OR _____
 (63) OPERATING RATING METHOD - _____
 (64) OPERATING RATING - _____
 (65) INVENTORY RATING METHOD - _____
 (66) INVENTORY RATING - _____
 (70) BRIDGE POSTING - _____
 (41) STRUCTURE OPEN, POSTED OR CLOSED - _____
 DESCRIPTION - _____

***** APPRAISAL *****

(67) STRUCTURAL EVALUATION _____
 (68) DECK GEOMETRY _____
 (69) UNDERCLEARANCES, VERTICAL & HORIZONTAL _____
 (71) WATERWAY ADEQUACY _____
 (72) APPROACH ROADWAY ALIGNMENT _____
 (36) TRAFFIC SAFETY FEATURES _____
 (113) SCOUR CRITICAL BRIDGES _____

***** PROPOSED IMPROVEMENTS *****

(75) TYPE OF WORK - _____ CODE _____
 (76) LENGTH OF STRUCTURE IMPROVEMENT _____ M
 (94) BRIDGE IMPROVEMENT COST \$ _____,000
 (95) ROADWAY IMPROVEMENT COST \$ _____,000
 (96) TOTAL PROJECT COST \$ _____,000
 (97) YEAR OF IMPROVEMENT COST ESTIMATE _____
 (114) FUTURE ADT _____
 (115) YEAR OF FUTURE ADT _____

***** INSPECTIONS *****

(90) INSPECTION DATE ____/____/____ (91) FREQUENCY ____ MO
 (92) CRITICAL FEATURE INSPECTION: (93) CFI DATE
 A) FRACTURE CRIT DETAIL - ____ - ____ MO A) ____/____
 B) UNDERWATER INSP - ____ - ____ MO B) ____/____
 C) OTHER SPECIAL INSP - ____ - ____ MO C) ____/____

Figure 4.1.2 Typical SI&A Sheet with NBI Data Only

Oregon Department of Transportation

Bridge Inspection Report

District	07	Structure	Coos Bay, Hwy 9 (McCullough)	Bridge ID	01823
Fac Crossed	COOS BAY (MCCULLOUGH BR)	Owner	State Highway Agency	Fac Carried	US101(HWY009)
Suff Rating	46.5	County	Coos	Mile Point	233.99mi
AC Depth	0.00	Record Type	1	Insp Date	06/11/2009
Bridge Length	5305.00ft	Insp Freq	24	Inspector 1	Jeff Swanstrom (2010)
		Bridge Width	33.80ft	Inspector 2	JOHN MILCAREK (241)

Signature: _____

		Element Condition States								
Elem	Description	Env	Qty	Units	1	2	3	4	5	status
18	Concrete Deck - Protected w/ Thin Overlay	Sev.	152100.00	sqft (SF)	0%	100%	0%	0%	0%	
110	Reinforced Conc Open Girder/Beam	Sev.	3332.00	ft (LF)	70%	20%	10%	0%	0%	
113	Painted Steel Stringer	Sev.	15372.00	ft (LF)	47%	48%	5%	0%	0%	
121	Painted Steel Bottom Chord Thru Truss	Sev.	3416.00	ft (LF)	47%	48%	5%	0%	0%	
126	Painted Steel Thru Truss (excl. bottom chord)	Sev.	3416.00	ft (LF)	45%	50%	5%	0%	0%	
144	Reinforced Conc Arch	Sev.	5522.00	ft (LF)	78%	20%	2%	0%	0%	
152	Painted Steel Floor Beam	Sev.	2090.00	ft (LF)	50%	48%	2%	0%	0%	
155	Reinforced Conc Floor Beam	Sev.	4862.00	ft (LF)	80%	15%	5%	0%	0%	
205	Reinforced Conc Column or Pile Extension	Sev.	64	(EA)	75%	20%	5%	0%	0%	
210	Reinforced Conc Pier Wall	Sev.	11	(EA)	20%	75%	5%	0%	0%	
215	Reinforced Conc Abutment	Sev.	2	(EA)	50%	50%	0%	0%	0%	
220	Reinforced Conc Submerged Pile Cap/Footing	Sev.	9	(EA)	100%	0%	0%	0%	0%	
221	Submerged Concrete Spread Footing	Sev.	2	(EA)	100%	0%	0%	0%	0%	
223	Submerged, Conc Footing Seal	Sev.	2	(EA)	100%	0%	0%	0%	0%	
234	Reinforced Conc Cap	Sev.	12	(EA)	80%	15%	5%	0%	0%	
304	Open Expansion Joint	Sev.	70.00	ft (LF)	50%	50%	0%	0%	0%	
305	Polychlorophrene Joint	Sev.	2552.00	ft (LF)	10%	20%	70%	0%	0%	
309	Other Joint	Sev.	3700.00	ft (LF)	100%	0%	0%	0%	0%	
310	Elastomeric Bearing	Sev.	8	(EA)	100%	0%	0%	0%	0%	
311	Moveable Bearing (roller, sliding, etc.)	Sev.	290	(EA)	35%	60%	5%	0%	0%	
313	Fixed Bearing	Sev.	4	(EA)	50%	50%	0%	0%	0%	
321	Reinforced Conc Approach Slab w/ or w/o AC Ovly	Sev.	2	(EA)	50%	50%	0%	0%	0%	
325	Traffic Impact Condition	Ben.	1	(EA)	0%	100%	0%	0%	0%	
326	Deck Wearing Surface	Ben.	1	(EA)	0%	100%	0%	0%	0%	
331	Reinforced Conc Bridge Railing	Sev.	7044.00	ft (LF)	90%	10%	0%	0%	0%	
334	Metal Bridge Railing - Coated	Sev.	1708.00	ft (LF)	70%	30%	0%	0%	0%	
357	Pack Rust	Sev.	1	(EA)	0%	0%	100%	0%	0%	
359	Soffit of Concrete Deck or Slab	Sev.	1	(EA)	38%	30%	30%	2%	0%	
363	Section Loss	Sev.	1	(EA)	100%	0%	0%	0%	0%	

Figure 4.1.3 Oregon Bridge Inspection Report with Element Level Data

390	Paint, Alkyd (incl red lead)	Sev.	3713.50sqft	(SF)	35%	60%	5%	0%	0%
990	Miscellaneous Items	Sev.	1	(EA)	100%	0%	0%	0%	0%
994	Miscellaneous Fender Sys Timber	Sev.	2	(EA)	0%	100%	0%	0%	0%

Appraisal			NBI Category		
Appraisal	NBI #	Rating	Category	NBI #	Rating
Scour	113	5 Stable w/in footing	Deck Condition	58	6 Satisfactory
Bridge Rail	36A	0 Substandard	Superstructure	59	5 Fair
Transitions	36B	0 Substandard	Substructure	60	6 Satisfactory
Approach Rail	36C	0 Substandard	Channel	61	7 Minor Damage
Rail Ends	36D	0 Substandard	Culvert/Retaining Walls	62	N N/A (NBI)
Structural	67	5 Above Min Tolerable			
Deck	68	3 Intolerable - Correct			
Clearance	69	N Not applicable (NBI)			
Waterway	71	9 Above Desirable			
Approach Alignment	72	8 Equal Desirable Crit			

Remarks

P Conc Deck/Thin Ovl (18)
 (6/09) Thin overlay overtops one of the joints.

R/Conc Open Girder (110)
 Bt 5 girder 1 has exposed stirrups (6/09)

P/Stl Thru Truss/Bot (121)
 (6/9) Lots of garbage/materials (PVC) on steel joints below the deck.

P/Stl Thru Truss/Top (126)
 (6/09) Missing rivets in SE spire at start of thru truss.

R/Conc Arch (144)
 CONCRETE ARCH'S HAVE HORIZONTAL CRACKS - NEAR THE CENTER TOP... (6/09) Steel exposed in spandrel column as well as cracks with efflor. on arches. Cathodic Protection project underway @ South approach spans

R/Conc Floor Beam (155)
 SOME OF THE CAPS, COLUMNS, HAVE CRACKS, SPALLS & EXPOSED REBAR

R/Conc Pier Wall (210)
 (6/09) Bt 7 pier wall, S. side, has corrosion cracking @ bottom of columns and delamination.

R/Conc Cap (234)
 MOST OF THE CAPS NEED WASHED... (6/09) Bent 7 cap has spalling w/exposed stirrup near column 2.

Open Expansion Joint (304)
 MANY OF THE JOINTS EDGES ARE SPALLING.ALL JOINTS ARE LEAKINGJOINT AT MIDSPAN HAS FAILED - PERCALL CORNER FAILED -

Other Joint (309)
 [none]

Moveable Bearing (311)
 (6/09) Verify total quantity of bearings after completion of cathodic protection.

Conc Bridge Railing (331)
 Concrete rail being replaced in south approach spans (6/09)

Misc (990)
 (6/09) Earthquake retrofit on S. end, bent 3, cables are tight

Fender System (994)
 UW report states rating for elem. 994 as CS1-95%, CS2-3%, and CS3-2%

Notes

Inspection Notes

Reviewed for Item #113, slays a T, jr, user #152, 09-02-08. Tidal hydraulics study needed to determine seriousness and extent of possible scour during the flood of maximum scour potential. Tidal hydraulics study done by West Consultants, changed item 113 from T to 5, 01-11-11, jr.

Figure 4.1.3 Oregon Bridge Inspection Report with Element Level Data (cont.)

BRIDGE GROUP

Structure Inventory & Appraisal

Structure Number: 4023		Structure Name: RCB		Feature Under: WASH	
Route: 60 MP 56.85		Road Name: US 60		Agency: ADOT	
				Location: 7.3 M E JCT SR 72	

LOCATION INFORMATION		DIMENSIONS		PROPOSED IMPROVEMENTS	
N1-State Code:	049	N32-Appr Rdwy Width (feet):	36	N75-Type of Work:	
N2-State Hwy District:	88	N48-Max Span Length (feet):	10	N76-Length of Str Imp (feet):	0
N3-County Code:	029	N49-Structure Length (feet):	32	N94-Br Improv Cost (x1000):	\$0
N4-Place Code:	00000	N50a-Lt Curb/Swtk Width (feet):	1	N95-Rdwy Improv Cost (x1000):	\$0
N16-Latitude:	33 deg 47.1 min	N50b-Rt Curb/Swtk Width (feet):	1	N96-Total Project Cost (x1000):	\$0
N17-Longitude:	113 deg 36.5 min	N51-Br Width Curb-Curb (feet):	39	N97-Year of Cost Estimate:	
N98-Border St Code - % Resp:	- 0	N52-Deck Width Out-Out (feet):	41.6		
N99-Border Bridge Number:		N112-NBIS Br Length?	Y		

INVENTORY ROUTE DATA		VERTICAL and HORIZONTAL CLEARANCE		CONSTRUCTION PROJECT DATA	
N19-Detour Length (miles):	20	N53-Min Vert Over Clr (feet):	99.99	N27-Year Built:	1958
N20-Toll:	3	N54-Min Vert Under Clr (feet):	N 0	N106-Year of Reconstruction:	0000
N28-Lanes On / Under:	2 / 0	N55-Min Lat Under Clr Rt (feet):	N 99.9	A204-Orig Project Number:	F-022-1(1)
	<i>2nd Record</i>	N56-Min Lat Under Clr Lt (feet):	0	A205-Orig Project Station:	3045+14.34
N5-Inv Rte:	1 2 0 00060 0 -			A223-TRACS Number:	
N10-Inv Rte Min Vert Clr (feet):	99.99			A225-Deck Area (sq. feet):	0
N11-Inv Rte Milepoint:	56.85			A226-Superstr Unit Cost:	\$0
N26-Functional Class:	07			A227-Substr Unit Cost:	\$0
N29-Avg Daily Traffic:	2417				
N30-Year of ADT:	1998				
N47-Inv Rte Tot Horiz Clr (feet):	39				
N100-Defense Hwy:	0				
N101-Parallel Bridge:	N				
N102-Direction of Traffic:	2				
N104-Hwy System:	0				
N109-Percent Truck Traffic:	46				
N110-National Truck Network:	1				
N114-Future ADT:	2427				
N115-Year of Future ADT:	2020				
N200-Is N5 the Princ. Rte?	Y				

SERVICE, TYPE, and SPAN INFORMATION		CONDITION RATINGS		INSPECTION	
N42-Service Type:	15	N58-Deck:	8	N90-Inspection Date:	2/1/2000
N43-Str Type, Main:	219	N59-Superstructure:	N	N91-Insp Freq (months):	48
N44-Str Type, Appr:	000	N60-Substructure:	N	A207-Inspection Quarter:	1
N45-Number of Main Spans:	3	N61-Channel:	7	A208-Inspection Number:	14
N46-Number of Appr Spans:	0	N62-Culvert:	7	A228-Next Insp Date:	Quarter 1, 2004

APPRAISAL RATINGS		CRITICAL FEATURES	
N67-Struct Evaluation:	7	N92A-Fracture Critical:	N 0
N68-Deck Geometry:	5	N92B-Underwater Insp:	N 0
N69-Underclearance Rtg:	N	N92C-Special Insp:	N 0
N71-Waterway Adequacy:	6	N93A-Date Fract Crit Insp:	0
N72-Appr Rdw Align:	8	N93B-Date Underwtr Insp:	0
N36-Traffic Safety Features:	0 0 0 0	N93C-Date Spec Insp:	0
		A234-Steel In-Depth Insp Freq (mo):	0

BRIDGE SCOUR DATA		CULVERT INFORMATION	
N113-Scour Critical Rtg:	8	A217-Culv Barrel Height (feet)	6
A202-Foundation Type:		A218-Culv Length (feet):	41
A220-Found Embed (feet):	0	A219-Culv Fill Height (feet):	1
A221-Scour Countermeasure:	0 1 0		

LOAD, RATE, and POST		BRIDGE RAILING	
N31-Design Loading:	5	A206a-Bridge Rail Type:	6
N41-Open, Post, Close:	A	A206b-Geometric Conform:	0
N63-Method Used for Oper. Rtg.:	5	A206c-Structural Conform:	0
N64-Operating Load Rtg:	2 - 36		
N65-Method Used for Inv. Rtg.:	5		
N66-Inventory Load Rtg:	2 - 36		
N70-Bridge Posting:	5		
N103-Temp Str Designation:			
A211-Posted Limit (Tons):	0		
A222-Date of Load Rtg:			
A233-Posted Vert Clr NB/EB (ft-in):	0 - 0		
A233-Posted Vert Clr SB/WB (ft-in):	0 - 0		

SUFFICIENCY RATING		GENERAL COMMENTS	
Sufficiency Rating:	92.32		

RESPONSIBILITY		NAVIGATION		GENERAL DATA	
I21-Maint Responsibility:	01	38-Navigation Control:	0	33-Bridge Median:	0
I22-Bridge Owner:	01	39-Nav Vert Clr (feet):	0	34-Skew:	0
.203-ADOT Org Number:	8852	40-Nav Horiz Clr (feet):	0	35-Structure Flared:	0
.224-Insp Team Number:	4	111-Nav Pier/Abut Prot:		37-Historical Significance:	5
.229-Agency:	ADOT	116-Nav Min Vert Clr (feet):	0	107-Deck Str Type:	1
				108-Wear Surf Prot System:	6 0 0
				201-Wear Surf Thickness (inches):	4

Figure 4.1.4 Arizona Structural Inventory and Appraisal Sheet

Structure ID: 520002

4 Description

Structure Unit Identification

Bridge/Unit ID 520002 0
 Description MAIN SPAN 1
 Type Main Span
 NBI Unit Flag Main Approach
 Curb/Sidewalk (50) Left 0 ft Right 0 ft
 Deck width (52) 0 ft
 Bridge Median (33) No median

Roadway Identification:

NBI Structure No (8) 520002
 Position/Prefix (5) Route On Structure
 Kind Hwy (Rte Prefix) U.S. Numbered Hwy
 Design Level of Service Mainline
 Route Number/Suffix 00090 / Not Applicable
 Feature Intersect (6) US90 SR10/GUM CREEK
 Critical Facility Not Defense-crit
 Facility Carried (7) US 90 SR 10
 Mile Point (11) 20.815
 Latitude (16) 030d47'39" Long (17) 085d43'28"

Roadway Classification

Nat. Hwy Sys (104) Not on NHS
 National base Net (12) On Base Network
 LRS Inventory Rte (13a) 52 010 000 Sub Rte (13b) 00
 Functional Class (26) Rural Minor Arterial
 Eligible for Federal Aid ? Yes
 Defense Hwy (100) Not a STRAHNET hwy
 Direction of Traffic (102) 2-way traffic
 Critical Travel Route

Structure Unit Type and Material

Struct Material (43) Concrete
 Design Type Culvert
 Deck Type (107) Not Applicable
 Surface (108) Not Applicable
 Membrane None
 Deck Protection None
 Skew (34) 0 deg

Roadway Traffic and Accidents

Lanes (28) 2 Medians 0 Speed 54.681 mph
 ADT Class ADT Class 3
 Recent ADT (29) 5100 Year (30) 1998
 Future ADT (114) 9490 Year (115) 2020
 Truck % ADT (109) 7
 Detour Length (19) 1.243 mi
 Detour Speed 44.739 mph
 Accident Count -1 Rate -1

Roadway Clearances

Vertical (10) 99.99 ft Appr. Road (32) 34.121 ft
 Horiz. (47) 34.121 ft Roadway (51) 0 ft
 Truck Network (110) Not part of natl network
 Toll Facility (20) On free road
 Fed. Lands Hwy (105) Not Applicable
 School Bus Route
 Transit Route

Figure 4.1.5 Florida Structural Inventory and Appraisal Sheet

REPORT ID: INVT001A

FLORIDA DEPARTMENT OF TRANSPORTATION
 BRIDGE MANAGEMENT SYSTEM
 COMPREHENSIVE INVENTORY DATA REPORT

Page 2 of 4

Structure ID: 520002

Structure Identification

Admin Area Not located in area
 District (2) D3 - Chipley
 County (3) (52)Holmes
 Place Code (4) No city involved
 Location (9) 3.2 KM W OF BONIFAY
 Border Br St/Reg (98) Not Applicable Share 0 %
 Border Struct No (99)
 FIPS State/Region (1) Florida Region 4-Atlanta
 NBIS Bridge Len (112) Meets NBI Length
 Parallel Structure (101) No II bridge exists
 Temp. Structure (103) Not Applicable
 Maint. Resp. (21) State Highway Agency
 Owner (22) State Highway Agency
 Historic Signif. (37) Not eligible for NRHP

Geometrics

Spans in Main Unit (45) 4
 Approach Spans (46) 0
 Length of Max Span (48) 9.843 ft
 Structure Length (49) 42.979 ft
 Deck Area -1 sqft
 Structure Flared (35) No flare

Age and Service

Year Built (27) 1954
 Year Reconstructed (106) -1
 Type of Service On (42a) Highway
 Under (42b) Waterway
 Fracture Critical Details Not Applicable

3 Appraisal

Structure Appraisal

Open/Posted/Closed (41) Open, no restriction
 Deck Geometry (68) Not Applicable
 Underclearances (69) Not Applicable
 Approach Alignment (72) No speed red thru curve
 Bridge Railings (36a) Not Applicable
 Transitions (36b) Not Applicable
 Approach Guardrail (36c) Meets Standards
 Approach Guardrail ends (36d) Meets Standards
 Scour Critical (113) Stable Above Footing

Navigation Data

Navigation Control (38) Permit Not Required
 Nav Vertical Clr (39) 0 ft
 Nav Horizontal Clr (40) 0 ft
 Min Vert Lift Clr (116) 0 ft
 Pier Protection (111) Not Applicable

NBI Condition Rating

Sufficiency Rating * 99.5
 Structural Eval (67) Above Min Criteria
 Deficiency Not Deficient

Minimum Vertical Clearance

Over Structure (53) 99.99 ft
 Under (reference) (54a) Feature not hwy or RR
 Under (54b) 0 ft

Minimum Lateral Underclearance

Reference (55a) Feature not hwy or RR
 Right Side (55b) 0 ft
 Left Side (56) 0 ft

Load Rating

Design Load (31) M 13.5 (H 15)
 Rating Date 08/08/1994 Initials JF
 Posting (70) At/Above Legal Loads

Operating Type (63) LF Load Factor
 Operating rating (64) 68.894 tons Alternate -1
 Inventory Type (65) LF Load Factor
 Inventory Rating (66) 40.896 tons Alternate -1
 Alt Meth -1

6 Schedule

Current Inspection

Inspection Date 01/06/2000
 Inspector MT338TK - Tom Klopfenstein
 Primary Type Regular NBI
 Review Required

Next Inspection Date Scheduled

NBI 01/06/2002
 Element 01/06/2002
 Fracture Critical
 Underwater
 Other Special

Inspection Types

Performed NBI Element Fracture Critical Underwater Other Special

Figure 4.1.5 Florida Structural Inventory and Appraisal Sheet (Continued)

Structure ID: 520002

Inspection Intervals	Required (92)	Frequency (92)	Last Date (93)	Inspection Resources
Fracture Critical	<input type="checkbox"/>	mos		Crew Hours 8
Underwater	<input type="checkbox"/>	mos		Flagger Hours 0
Other Special	<input type="checkbox"/>	mos		Helper Hours 0
NBI		24 mos (91)	01/06/2000 (90)	Snooper Hours 0
				Special Crew Hours 0
				Special Equip Hours 0

5 Custom

General Bridge Information

Parallel Bridge Seq	Bridge Rail 1 Not applicable-No rail
Channel Depth 0.328 ft	Bridge Rail 2 Not applicable-No rail
Radio Frequency -1	Electrical Devices No electric service
Phone Number (000) 000-0001	Culvert Type Not applicable
Exception Date	Maintenance Yard Marianna Yard
Exception Type Unknown	

Bridge Load Rating Information

Govr. Span Length 9.843 ft	Single Unit Truck 2 Axles 48.502 tons
L-Rating Origination Design Plans	Single Unit Truck 3 Axles 60.627 tons
Load Rating Date 08/08/1994	Single Unit Truck 4 Axles 74.957 tons
Method Calculation AASHTO formula	Combination Unit Truck 3 Axles 79.366 tons
Load Dist. Factor 0.168	Combination Unit Truck 4 Axles 79.366 tons
Impact Factor 0	Combination Unit Truck 5 Axles 87.083 tons
Design Method Load Factor	Truck Trailer 5 Axles 95.901 tons
Design Measure English	Posting Weight tons
Recommended Single Unit -1 tons	Posting Single Unit -1 tons
Recommended Combination -1 tons	Posting Combination Unit -1 tons
Recommended Tandem -1 tons	Posting Tandem Unit -1 tons

Bridge Scour and Storm Information

Pile Driving Record Not Applicable	Scour Recommended I Stop scour evaluations
Foundation Type Foundation details	Scour Recommended II Unknown
Mode of Flow Riverine	Scour Recommended III Unknown
Rating Scour Eval Low Risk - Low	Scour Elevation -1 ft
Highest Scour Eval Phase I completed	Action Elevation -1 ft
	Storm Frequency -1

1 Condition

NBI Rating

Channel (61) No Deficiencies	Culvert (62) Minor Deterioration
Deck (58) Not Applicable	Waterway (71) 8 - Equal Desirable
Superstructure (59) Not Applicable	Unrepaired Spalls -1 sq.ft.
Substructure (60) Not Applicable	Review Required <input type="checkbox"/>

Figure 4.1.5 Florida Structural Inventory and Appraisal Sheet (Continued)

REPORT ID: INVT001A

FLORIDA DEPARTMENT OF TRANSPORTATION
 BRIDGE MANAGEMENT SYSTEM
 COMPREHENSIVE INVENTORY DATA REPORT

Page 4 of 4

Structure ID: 520002

Elements

Inspection Date: 01/06/2000 GKXW

Span Id	Elem/ErDescription	Qty1	%1	Qty2	%2	Qty3	%3	Qty4	%4	Qty5	%5	T Qty
0	290/4 Channel	1	100	0	0	0	0	0	0	0	0	1 ea.

Notes

0	475/4 R/Conc Walls	154	100	0	0	0	0	0	0	0	0	154 lf.
---	--------------------	-----	-----	---	---	---	---	---	---	---	---	---------

Notes

0	241/4 Concrete Culvert	299	82	66	18	0	0	0	0	0	0	364 lf.
---	------------------------	-----	----	----	----	---	---	---	---	---	---	---------

Notes There are a few vertical cracks in the side walls of the original section of culvert.

Total Number of Elements: 3

Past Inspections

Inspection Date: 01.06.2000

Type: Regular NBI

Inspector: MT338TK - Tom Klopfenstein

Inspection Notes: Sufficiency Rating Calculation Accepted by mt338tk at 01/10/2000 13:45:43
 MT338TK inspection comments - The left extended portion of culvert is skewed 24 degrees to the left due to stream alignment.
 Structure 520002 -
 Date 01/06/2000 -
 Previous comments > (none)

Inspection Date: 04.01.1998

Type: Regular NBI

Inspector: BID

Inspection Notes:

Bridge Notes

Figure 4.1.5 Florida Structural Inventory and Appraisal Sheet (Continued)

Some agencies furnish standardized sketch sheets and photo sheets to inspectors for report generation. Some agencies have developed their forms on software packages for use on portable computers (see Figures 4.1.6 and 4.1.7) or wearable computers (see Figures 4.1.8 and 4.1.9).



Figure 4.1.6 Portable Computer



Figure 4.1.7 Inspector Using Portable Computer



Figure 4.1.8 Wearable Computer with Case



Figure 4.1.9 Inspector Using Wearable Computer

The data and information required of states by the FHWA is listed in the *FHWA Coding Guide* and *AASHTO Manual for Bridge Evaluation*. It is important to note that several items listed in the *FHWA Coding Guide* apply to both the field and office personnel responsible for bridge inspections. The bridge inspector is typically not required to obtain the data for all the items during every inspection of a bridge. Once a bridge has been inventoried, the majority of the geometric and other inventory items will remain unchanged. The inspector is responsible for spot checking to see if inventoried items are consistent with observations at the bridge site.

4.1.3

Inventory Items

Inventory items pertain to a bridge's characteristics. For the most part, these items are permanent characteristics, which only change when the bridge is altered in some way, such as reconstruction or load restriction. Inventory items include the following SI&A items:

- Identification – Identifies the structure using location codes and descriptions.
- Structure Type and Material – Categorizes the structure based on the material, design and construction, the number of spans, and wearing surface.
- Age and Service – Information showing when the structure was constructed or reconstructed, features the structure carries and crosses, and traffic information.
- Geometric Data – Includes pertinent structural dimensions.
- Navigation Data – Identifies the existence of navigation control, pier protection, and waterway clearance measurements.
- Classification – Classification of the structure and the facility carried by the structure are identified.
- Load Rating and Posting – Identifies the load capacity of the bridge and the current posting status. This item is subject to change as conditions change and is therefore not viewed as a "permanent" item.
- Proposed Improvements – Items for work proposed and estimated costs for all bridges eligible for funding from the Highway Bridge Program.
- Inspection – Includes latest inspection dates, designated frequency, and critical features requiring special inspections or special emphasis during inspection.

All inventory items are explained in the *FHWA Coding Guide*. Although inventory items are usually provided from previous reports, the inspector is responsible for verifying and updating the inventory data as needed. See Topic 4.2 for condition and appraisal rating items.

4.1.4

Condition and Appraisal Rating Items

Condition Rating Items Condition ratings are used to describe the existing, in-place bridge as compared to the as-built condition. Condition ratings are typically coded by the inspector. Condition rating items include:

- Deck – Describes the overall condition rating of the deck. This condition of the surface/protective systems, joints, expansion devices, curbs, sidewalks, parapets, fascias, bridge rail and scuppers is not included in the rating, but the condition will be noted in the inspection form. Decks that are integral with the superstructure will be rated as a deck only and not influence the superstructure rating.
- Superstructure – Describes the physical condition of all the structural members. The condition of the bearings, joints, paint system, etc. will not be included in the rating except for extreme situations, but the condition will be noted in the inspection form. Superstructures that are integral with the deck will be rated as a superstructure only and not influence the deck rating.
- Substructure – Describes the physical condition of piers, abutments, piles, fenders, footings or other components.
- Channel and channel protection – Describes the physical condition that is associated with the flow of the water through the bridge which include the stream stability and the condition of the hydraulic countermeasures.
- Culvert – Evaluates the alignment, settlement, joints, structural condition, scour and any other of the items that may be associated with a culvert.

Appraisal Rating Items Condition ratings are a judgment of a bridge component condition in comparison to current standards. Appraisal items are used to evaluate a bridge in relation to the level of service which it provides on the highway system of which it is a part. The structure will be compared to a new one which is built to current standards for that particular type of road. Appraisal rating items include:

- Structural Evaluation – Overall evaluation of the structure based on the lowest bridge component condition rating, excluding the deck, superstructure, substructure, channel and channel protection and culverts. This item is calculated by the FHWA Edit/Update program.
- Deck Geometry – Evaluates the curb-to-curb bridge roadway width and the minimum vertical clearance over the bridge roadway. This item is calculated by the FHWA Edit/Update program.
- Under-clearances, Vertical and Horizontal – The vertical and horizontal under-clearances from the through roadway under the structure to the superstructure or substructure units. This item is calculated by the FHWA Edit/Update program.
- Waterway Adequacy – Appraises waterway opening with respect to passage of flow under the bridge.
- Approach Roadway Alignment – Comparing the alignment of the bridge

approaches to the general highway alignment of the section of highway that the structure is on.

- Traffic Safety Features – Record information on bridge railings, transitions, approach guiderail, approach guiderail ends, so that evaluation of their adequacy can be made.
- Scour Critical Bridges – Identify the current status of the bridge regarding its vulnerability to scour.

4.1.5

The Role of Inventory Items in Bridge Management Systems

Inventory items are an important part of an owner's Bridge Management System (BMS). Bridge owners use the inventory items to help plan inspection, maintenance, and reconstruction of their bridges, as well as classify their bridges. There have been times when there has been a problem on a particular bridge and the owners used the inventory items of that bridge to search for the same potential problems that might exist on other bridges.

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Topic 4.2 Condition and Appraisal

4.2.1

Introduction

The reported condition of an element or component is an evaluation of its current physical state compared to what it was on the day it was built. Appraisal rating items are used to evaluate a bridge in relation to the level of service it provides on the highway system of which it is a part.

4.2.2

Condition Rating Items

Deck, Superstructure and Substructure

Accurate assignment of condition ratings is dependent upon the bridge inspector's ability to identify the bridge components and their elements. Bridge components are the major parts comprising a bridge including the deck, superstructure, and substructure. Bridge elements are individual members comprised of basic shapes and materials connected together to form bridge components.

The overall condition rating of bridge components is directly related to the physical deficiencies of bridge elements.

Evaluating Elements

The inspector is responsible for evaluating each element of each component and assigning to it a descriptive condition rating of "good," "fair," or "poor," based on the physical deficiencies found on the individual element. The following guidelines are used in establishing an element's condition rating:

- Good - element is limited to only minor problems.
- Fair - structural capacity of element is not affected by minor deterioration, section loss, spalling, cracking, or other deficiency.
- Poor - structural capacity of element is affected or jeopardized by advanced deterioration, section loss, spalling, cracking, or other deficiency.

To ensure a comprehensive inspection and as a part of the requirements of record keeping and documentation, an inspector is responsible for recording the location, type, size, quantity, and severity of deterioration and deficiencies for each element of a given component.

Evaluating Components

The following major components of bridges receive an overall Structure Inventory and Appraisal (SI&A) component condition rating:

- Item No. 58 – Deck
- Item No. 59 – Superstructure
- Item No. 60 – Substructure

Component Condition Rating Guidelines

NBI component condition ratings for deck, superstructure, or substructure components, in general, should reflect the overall condition of the component rather than localized conditions. This has been true for many years and is emphasized in the FHWA *Coding Guide* with the following wording:

Condition codes are properly used when they provide an overall characterization of the general condition of the entire component being rated. Conversely, they are improperly used if they attempt to describe localized or nominally occurring instances of deterioration or disrepair. Correct assignment of a condition code must, therefore, consider both the severity of the deterioration or disrepair and the extent to which it is widespread throughout the component being rated.

Although the *FHWA Coding Guide* states that it is improper to use the condition codes to describe localized instances of deterioration or disrepair, it also states that the inspector must consider both the severity and extent of the deterioration. With this in mind, there are occasions when a severe, localized condition affects the structural capacity of a component member. It is important to recognize that the coding applies to all primary members of a component. Therefore, localized conditions that impact the structural capacity of just one member can impact the overall performance of the entire component. The affect on structural capacity is dependent upon several factors including the type and extent of the deterioration, as well as the location along the member. An inspector may need to discuss the observed condition with an engineer to make this determination. When these situations occur, it is appropriate to assign a lower component condition rating for that component from a safety perspective and is in keeping with the intent of the National Bridge Inspection Program.

When these localized conditions are determined to be such that prompt action is needed and/or the overall component condition rating is affected, the conditions should also be addressed through the "critical findings" process that is identified in the NBIS regulation. The NBI component condition rating should be reviewed and appropriately adjusted once the critical finding has been addressed. This adjustment will depend on how the critical finding was addressed and how that action relates to the original rating rationale.

The coding of NBI condition items should be viewed as important, but secondary, to the recognition of and follow-up on critical findings.

Currently, states employ two approaches to coding condition items when localized areas of severe deterioration are encountered. Some will account for the severity of a localized area of deterioration by lowering the condition rating of an entire component. The component condition rating is adjusted after the deteriorated area is improved (i.e., rating may rise if physical improvements are made, or may stay the same if the bridge is posted for load restrictions and/or supported with temporary shoring). FHWA recognizes this approach when the severity of the localized deterioration affects the structural capacity of the component.

Other states "rate to the average" regardless of the severity of a localized area of deterioration. This approach relies heavily on ensuring that critical findings are addressed in a timely manner regardless of the component condition rating value. If the localized area of severe deterioration is not improved following the critical finding follow-up process, the component condition rating may need to be lowered to account for the severity of the deterioration if structural capacity is affected.

Either approach to coding the condition items results in the same ultimate outcome, i.e. critical inspection findings are addressed to ensure continued safe use

of the bridge and component condition ratings eventually reflect the overall condition of the component. If the approach is to consider both the severity and extent of a component's deterioration in rating each component at the time of inspection (or up to 90 days after the inspection as required by the NBIS), there cannot be any assumptions about future improvements made to a localized area. Only if an improvement is made, the rating should then be raised as appropriate. If the improvement is made within 90 days of the inspection, there is no need to consider the localized deterioration in the rating.

The following general component condition rating guidelines (obtained from the 1995 edition of the *FHWA Coding Guide*) are to be used in the evaluation of the deck (Item 58), superstructure (Item 59), and substructure (Item 60):

<u>Code</u>	<u>Description</u>
N	NOT APPLICABLE
9	EXCELLENT CONDITION
8	VERY GOOD CONDITION - no problems noted.
7	GOOD CONDITION - some minor problems.
6	SATISFACTORY CONDITION - structural elements show some minor deterioration.
5	FAIR CONDITION - all primary structural elements are sound but may have minor section loss, cracking, spalling, or scour.
4	POOR CONDITION - advanced section loss, deterioration, spalling, or scour.
3	SERIOUS CONDITION - loss of section, deterioration, spalling, or scour have seriously affected primary structural components. Local failures are possible. Fatigue cracks in steel or shear cracks in concrete may be present.
2	CRITICAL CONDITION - advanced deterioration of primary structural elements. Fatigue cracks in steel or shear cracks in concrete may be present or scour may have removed substructure support. Unless closely monitored it may be necessary to close the bridge until corrective action is taken.
1	"IMMINENT" FAILURE CONDITION - major deterioration or section loss present in critical structural components, or obvious vertical or horizontal movement affecting structure stability. Bridge is closed to traffic but corrective action may put bridge back in light service.
0	FAILED CONDITION - out of service; beyond corrective action.

The component condition rating guidelines presented above are general in nature and can be applied to all bridge components and material types.

Structural capacity is defined as the designed strength of the member. However, structural capacity is different than load-carrying capacity. Load-carrying capacity refers to the ability of the member to carry the legal loads of the highway system of which the bridge is a part. Therefore, a bridge could possibly have good structural capacity yet be load posted because it is unable to carry the legal loads.

A bridge's load-carrying capacity is not to influence component condition ratings. The fact that a bridge was designed for less than current legal loads, and may even be posted, has no influence upon component condition ratings.

Component condition ratings are determined by applying condition descriptions, which are general in nature, covering a broad array of bridge components and material types. The inspector is responsible for being familiar with terminology concerning material types and associated deficiency to utilize condition descriptions for accurately assigning component condition ratings. The following illustrates several common deficiency terms found in condition descriptions and their associated material types:

- Section loss usually applies to steel members or reinforcing steel
- Fatigue crack applies to steel members
- Cracking/spalling usually are used to describe concrete
- Shear crack usually applies to concrete but may apply to timber as well
- Checks/splits applies to timber members
- Scour can apply to substructure

Establishing a link between material type and deficiency allows for accurate component condition ratings determined by utilizing condition descriptions for ratings 9 through 1 found in the general component condition rating guidelines.

Supplemental component condition rating guidelines, which may be developed by individual states, are intended to be used in addition to the *FHWA Coding Guide* to make it easier for the inspector to assign the most appropriate condition rating to the component being considered and improve uniformity.

Using the material and component specific supplemental rating guidelines (found in the 1995 edition of the *FHWA Coding Guide*) helps to clarify how each type of deficiency affects the component condition rating. Care has to be taken not to "pigeonhole" the rating based on only one word or phrase. The following is one suggested method for determining proper component condition ratings:

- Identify phrases that describe the component
- Read through the rating scale until encountering phrases that describe conditions that are more severe than what actually exists
- Be sure to read down the ratings list far enough
- Correct rating number then is one number higher

This procedure generally works with all of the component condition rating guidelines.

4.2.3

Channel and Channel Protection Condition Ratings

General

For structures located over waterways, a Structure Inventory and Appraisal (SI&A) condition rating is provided for the channel and channel protection:

- Item No. 61 – Channel and Channel Protection

Overall Condition

This item describes the physical conditions associated with the flow of water through the bridge such as stream stability and the condition of the channel, riprap, slope protection, or stream control devices, including spur dikes. The inspector should be particularly concerned with visible signs of excessive water velocity which may cause undermining of slope protection, erosion of banks, and realignment of the stream. Accumulation of drift and debris on the superstructure and substructure should be noted on the inspection form but not included in the component condition rating of the superstructure and substructure.

Evaluate and code the condition in accordance with the previously described general component condition ratings, procedures to account for critical findings, and the following descriptive codes:

Code Description

- | | |
|---|--|
| N | Not applicable. Use when bridge is not over a waterway (channel). |
| 9 | There are no noticeable or noteworthy deficiencies which affect the condition of the channel. |
| 8 | Banks are protected or well vegetated. River control devices such as spur dikes and embankment protection are not required or are in a stable condition. |
| 7 | Bank protection is in need of minor repairs. River control devices and embankment protection have a little minor deficiency. Banks and/or channel have minor amounts of drift. |
| 6 | Bank is beginning to slump. River control devices and embankment protection have widespread minor deficiency. There is minor streambed movement evident. Debris is restricting the channel slightly. |
| 5 | Bank protection is being eroded. River control devices and/or embankment have major deficiency. Trees and brush restrict the channel. |
| 4 | Bank and embankment protection is severely undermined. River control devices have severe deficiency. Large deposits of debris are in the channel. |
| 3 | Bank protection has failed. River control devices have been destroyed. Streambed aggradation, degradation, or lateral movement has changed the channel to now threaten the bridge and/or approach roadway. |
| 2 | The channel has changed to the extent the bridge is near a state of collapse. |
| 1 | Bridge closed because of channel failure. Corrective action may put bridge back in light service. |

0 Bridge closed because of channel failure. Replacement necessary.

4.2.4

Culvert Condition Ratings

General

When assigning a culvert condition rating, all areas of the culvert and the possible effects on the overall structure are investigated. The inspector considers whether the component is functioning properly, whether it could pose a threat to safety or cause property damage, and whether it could cause more extensive damage if not repaired.

Evaluating Elements

Chapter 14 addresses the individual elements of various culverts. The overall component condition rating considers all of the elements which make up a culvert and are useful in establishing maintenance, rehabilitation, and replacement programs and priorities.

Although some of the individual elements of culverts are not directly considered in the *FHWA Coding Guide*, these supplemental items are useful in determining the overall culvert condition ratings. They may also be included as part of an agency's bridge management system.

Evaluating Components

In addition to the major components of bridges (deck, superstructure, and substructure), culverts also receive a Structure Inventory and Appraisal (SI&A) overall component condition rating:

➤ Item No. 62 – Culverts

Component Condition Rating Guidelines

This item evaluates the alignment, settlement, joints, structural condition, scour, and other items associated with culverts. The component condition rating code is intended to be an overall condition evaluation of the culvert. Integral wingwalls to the first construction or expansion joint are included in the evaluation.

Item 58 – Deck, Item 59 – Superstructure, and Item 60 – Substructure should be coded N for all culverts.

Evaluate and code the culvert condition in accordance with the previously described general component condition ratings, procedures to account for critical findings and the following descriptive codes:

<u>Code</u>	<u>Description</u>
-------------	--------------------

N	Not applicable. Use if structure is not a culvert.
9	No deficiencies.
8	No noticeable or noteworthy deficiencies which affect the condition of the culvert. Insignificant scrape marks caused by drift.
7	Shrinkage cracks, light scaling, and insignificant spalling which does not expose reinforcing steel. Insignificant damage caused by drift with no misalignment and not requiring corrective action. Some minor scouring

- has occurred near curtain walls, wingwalls, or pipes. Metal culverts have a smooth symmetrical curvature with superficial corrosion and no pitting.
- 6 Deterioration or initial disintegration, minor chloride contamination, cracking with some leaching, or spalls on concrete or masonry walls and slabs. Local minor scouring at curtain walls, wingwalls, or pipes. Metal culverts have a smooth curvature, non-symmetrical shape, significant corrosion, or moderate pitting.
 - 5 Moderate to major deterioration or disintegration, extensive cracking and leaching, or spalls on concrete or masonry walls and slabs. Minor settlement or misalignment. Noticeable scouring or erosion at curtain walls, wingwalls, or pipes. Metal culverts have significant distortion and deflection in one section, significant corrosion or deep pitting.
 - 4 Large spalls, heavy scaling, wide cracks, considerable efflorescence, or opened construction joint permitting loss of backfill. Considerable settlement or misalignment. Considerable scouring or erosion at curtain walls, wingwalls, or pipes. Metal culverts have significant distortion and deflection throughout, extensive corrosion or deep pitting.
 - 3 Any condition described in Code 4 but which is excessive in scope. Severe movement or differential settlement of the segments, or loss of fill. Holes may exist in walls or slabs. Integral wingwalls nearly severed from culvert. Severe scour or erosion at curtain walls, wingwalls, or pipes. Metal culverts have extreme distortion and deflection in one section, extensive corrosion, or deep pitting with scattered perforations.
 - 2 Integral wingwalls collapsed, severe settlement of roadway due to loss of fill. Section of culvert may have failed and can no longer support embankment. Complete undermining at curtain walls and pipes. Corrective action required to maintain traffic. Metal culverts have extreme distortion and deflection throughout with extensive perforations due to corrosion.
 - 1 Bridge closed. Corrective action may put bridge back in light service.
 - 0 Bridge closed. Replacement necessary.

4.2.5

Appraisal Rating Items

Appraisal Rating Guidelines

The following SI&A items are known as appraisal rating items:

- Item No. 67 – Structural Evaluation
- Item No. 68 – Deck Geometry
- Item No. 69 – Underclearances, Vertical and Horizontal
- Item No. 71 – Waterway Adequacy
- Item No. 72 – Approach Roadway Alignment
- Item No. 36 – Safety Features
- Item No. 113 – Scour Critical Bridges

Appraisal rating items are used to evaluate a bridge in relation to the level of

service it provides on the highway system of which it is a part. The level of service for a bridge describes the function the bridge provides for the highway system carried by the bridge. The structure is compared to a new one that is built to current standards for that particular class of road. The exception is Item 72, Approach Roadway Alignment. Rather than comparing the alignment to current standards, it is compared to the general existing alignment of the roadway approaches to the bridge compared to the general highway.

The level of service goals used to appraise bridge adequacy vary depending on the highway functional classification, traffic volume, and other factors. The goals are set with the recognition that widely varying traffic needs exist throughout highway systems. Many bridges on local roads can adequately serve traffic needs with lower load capacity and geometric standards than would be necessary for bridges on heavily traveled main highways.

If national uniformity and consistency are to be achieved, similar structure, roadway, and vehicle characteristics are evaluated using identical standards. Therefore, tables and charts have been developed which are used to evaluate the appraisal rating items for all bridges submitted to the National Bridge Inventory, regardless of individual state criteria used to evaluate bridges.

The following general appraisal rating guidelines (obtained from the 1995 edition of the *FHWA Coding Guide*) are used to evaluate structural evaluation (Item 67), deck geometry (Item 68), underclearances (Item 69), waterway adequacy (Item 71) and approach roadway alignment (Item 72).

Code Description

N	Not applicable
9	Superior to present desirable criteria
8	Equal to present desirable criteria
7	Better than present minimum criteria
6	Equal to present minimum criteria
5	Somewhat better than minimum adequacy to tolerate being left in place as is
4	Meets minimum tolerable limits to be left in place as is
3	Basically intolerable, requiring high priority of corrective action
2	Basically intolerable, requiring high priority of replacement
1	This value of rating code not used
0	Bridge closed

The specific tables for Item 67 - Structural Evaluation, Item 68 - Deck Geometry, Item 69 - Underclearances, Vertical and Horizontal, Item 71 - Waterway Adequacy and Item 72 - Approach Roadway Alignment appear in the *FHWA Coding Guide* and are detailed enough that several states now program their computerized bridge management system to automatically calculate several of the appraisal rating items. Thus, some inspectors may not be responsible for coding these items. Inspectors may be asked to field verify the computed appraisal ratings.

Item 67 - Structural Evaluation - The item description and procedures used to determine the Structural Evaluation Appraisal Rating are located in Item 67 of the *FHWA Coding Guide*. This item is coded by the FHWA Edit/Update program, not the inspector. The correct way to evaluate this item for bridges is to consider the following factors:

- The lowest rating dictated by Item 59 - Superstructure, Item 60 - Substructure or Comparison of Item 29 - ADT and Item 66 - Inventory Rating.
- For culverts, the lower of Item 62 - Culverts or Comparison of Item 29 - ADT and Item 66 - Inventory Rating.
- Appraisal codes of 3 or less can be achieved without the superstructure and substructure controlling with the comparison of Item 29 – ADT and Item 66 – Inventory rating

Item 68 - Deck Geometry - The deck geometry appraisal evaluates the curb to curb bridge roadway width and the minimum vertical clearance over the bridge roadway. This item is coded by determining two appraisal ratings, one for bridge roadway width and one for the minimum vertical clearance. The lower of these two is the appraisal rating. This item is coded by the FHWA Edit/Update program, not the inspector. The *FHWA Coding Guide* includes the following scenarios to choose from for the bridge roadway width appraisal:

- Bridges with two lanes carrying two-way traffic.
- Bridges with one lane carrying two-way traffic.
- All other two-way traffic situations.
- Bridges with one-way traffic.

Item 69 - Underclearances, Vertical and Horizontal - This item refers to the vertical and horizontal underclearances from the through roadway under the structure to the superstructure or substructure units. The item description and coding guidelines, which are located in Item 69 of the *FHWA Coding Guide*, are used to determine the Underclearance Appraisal Rating. This item is similar to Item 68 in that two different ratings are developed: one for vertical underclearance and one for horizontal underclearance. The lower of these two is the appraisal rating. This item is coded by the FHWA Edit/Update program, not the inspector.

Item 71 - Waterway Adequacy - Waterway adequacy is appraised with respect to passage of flow through the bridge. The rating is tied to flood frequencies and traffic delays. Appraisal ratings are assigned by the table contained in Item 71 of the *FHWA Coding Guide* and are based on the functional classification of the road carried by the structure, hydraulic and traffic data for the structure, and site conditions. This item is not coded by the FHWA Edit/Update program.

Item 72 - Approach Roadway Alignment – This appraisal is based on comparing the alignment of the bridge approaches to the general highway alignment of the section of roadway on which the structure is located. The rating guidelines are correctly applied by determining if the vertical or horizontal curvature of the bridge approaches differs from the section of highway the bridge is on, resulting in a reduction of vehicle operating speed to cross the bridge. This item is not coded by the FHWA Edit/Update program. The guidelines for FHWA Item 72, Appraisal or

Approach Roadway Alignment, are as follows:

- If no reduction in the operating speed of a vehicle is required compared to the highway, code Item 72 as an “8.”
- If only a very minor reduction in the operating speed of a vehicle is required compared to the highway, code Item 72 as a “6.”
- If a substantial reduction in the operating speed of a vehicle is required compared to the highway, code Item 72 as a “3.”

The following guidelines indicate a means of determining the difference between a minor reduction and substantial reduction of operating speed:

- Minor reduction in operating speed - ≤ 9 mph
- Substantial reduction in operating speed - ≥ 10 mph

The remaining codes between these general values are applied at the inspector's discretion.

A narrow bridge does not affect the Approach Roadway Alignment Appraisal. The narrow bridge would be accounted for in Item 68, Deck Geometry.

Items affecting sight distance at the bridge, unrelated to vertical and horizontal curvature of the roadway, such as vegetation growth and substructure units of overpass structures do not affect the Approach Roadway Alignment Appraisal.

Item 36 - Traffic Safety Features - For structures on the National Highway System (NHS), this appraisal is based on comparing the traffic safety features in place at the bridge site to current national standards set by regulation, so that an evaluation of their adequacy can be made. For structures not on the National Highway System (NHS), the procedure is the same, however, it shall be the responsibility of the highway agency (state, county, local, or federal) to set standards. The item description and procedures used to determine the Traffic Safety Feature Appraisal Rating are located in Item 36 of the *FHWA Coding Guide*. The following are the traffic safety features to be coded:

- Bridge Railings
- Transitions
- Approach Guiderail
- Approach Guiderail Ends

Item 113 - Scour Critical Bridges – This item is used to identify the current status of the bridge regarding its vulnerability to scour. A scour critical bridge is one with abutment or pier foundations that are rated as unstable due to observed scour at the bridge site, or a scour potential as determined from a scour evaluation study including a scour analysis made by hydraulic, geotechnical, or structural engineers. The item description, procedures, and code descriptions are located in Item 113 of the *FHWA Coding Guide*.

4.2.6

Functionally Obsolete and Structurally Deficient

Definitions

A bridge is considered to be functionally obsolete if it has deck geometry, load carrying capacity, clearance or approach roadway alignment that no longer meets the criteria for the system of which the bridge is a part. Examples include bridges with inadequate lane widths or shoulder widths, insufficient vertical clearances to serve the traffic demand, or bridges that may be occasionally flooded.

Bridges are considered structurally deficient where significant load carrying elements are found to be in poor or worse condition due to deterioration and/or damage, or the adequacy of the waterway opening provided by the bridge is determined to be extremely insufficient to the point of causing intolerable traffic interruptions.

Any bridge classified as structurally deficient is excluded from the functionally obsolete category. Bridges that are structurally deficient and functionally obsolete are reported together as deficient bridges.

General Qualifications

In order to be considered for either the structurally deficient or functionally obsolete classification, a highway bridge must meet the following:

Structurally Deficient (SD) -

1. A condition rating of 4 or less for
 - Item 58 - Deck; or
 - Item 59 - Superstructures; or
 - Item 60 - Substructures; or
 - Item 62 - Culvert and Retaining Walls.⁽¹⁾ or
2. An appraisal rating of 2 or less for
 - Item 67 - Structural Evaluation; or
 - Item 71 - Waterway Adequacy.⁽²⁾

Functionally Obsolete (FO) -

1. An appraisal rating of 3 or less for
 - Item 68 - Deck Geometry; or
 - Item 69 - Underclearances;⁽³⁾ or
 - Item 72 - Approach Roadway Alignment. or
2. An appraisal rating of 3 for
 - Item 67 - Structural Evaluation; or
 - Item 71 - Waterway Adequacy.⁽²⁾

Footnotes for structurally deficient and functionally obsolete:

- (1) Item 62 applies only if the last digit of Item 43 (Structure Type) is coded 19.
- (2) Item 71 applies only if the last digit of Item 42 (Type of Service) is coded 0, 5, 6, 7, 8 or 9.
- (3) Item 69 applies only if the last digit of Item 42 is coded 0, 1, 2, 4, 6, 7 or 8.

4.2.7

Sufficiency Rating

Definition

Sufficiency rating (S.R.) is a calculated numeric value used to indicate the sufficiency of a bridge to remain in service. The rating is calculated using the sufficiency rating formula. Sufficiency rating is discussed in detail in Appendix B of the *FHWA Coding Guide*.

Sufficiency Rating Formula

$$S.R. = S_1 + S_2 + S_3 - S_4$$

$$0 \leq S.R. \leq 100$$

(entirely deficient) (entirely sufficient)

where: S_1 = 55% max.; based on structural adequacy and safety (i.e., superstructure, substructure or culvert condition and load capacity).

S_2 = 30% max.; deals with serviceability and functional obsolescence (items such as deck condition, structural evaluation, deck geometry, underclearances, waterway adequacy, approach road alignment).

S_3 = 15% max.; concerns essentiality for public use (items such as detour length, average daily traffic, and STRAHNET (Strategic Highway Corridor Network)).

S_4 = 13% max.; deals with special reductions based on detour length, traffic safety features, and structure type.

Twenty NBI items are used to calculate these four factors which therefore determine the sufficiency rating. Sufficiency rating is not normally calculated manually. Usually, it is included in the agency's inventory computer program and is calculated automatically by the computer based upon the inventory data collected by the bridge inspector. The sufficiency rating is calculated by the FHWA Edit/Update program.

Uses

Sufficiency Rating (SR) is used by the federal and state agencies to determine the relative sufficiencies of all of the nation's bridges. In the recent past, eligibility for federal funding with Highway Bridge Program funds has been determined by the following criteria:

$$S.R. \leq 80 \quad \text{Eligible for rehabilitation}$$

S.R. < 50 Eligible for replacement

Some states use the sufficiency rating as the basis for establishing priority for repair or replacement of bridges; the lower the rating, the higher the priority. Several states have developed specific bridge management procedures with priority guidelines for repair or replacement of bridges. By using these types of procedures, priority ratings can be established by considering the significance or impact of such level-of-service parameters as traffic volume and class of highway.

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Topic 4.3 Introduction to Element Level Evaluation

4.3.1

Introduction

Managers of large inventories of infrastructure assets need a tool to effectively manage these assets. For bridge data, element level inspection has been successfully used as a basis for data collection, performance measurement, resource allocation, and management decision support. Although component condition rating and reporting, as described in the FHWA *Coding Guide*, provides a consistent method for evaluation and reporting, the data is not comprehensive enough to support bridge preservation performance-based decision support.

The Pontis CoRe (Commonly Recognized) Element Report (June 1993), which is the basis of the AASHTO CoRe Element Guide, was prepared by technical working group representatives from California, Colorado, Minnesota, Oregon, Virginia, Washington, and the Federal Highway Administration. The Pontis CoRE Report explains the reasoning behind the selection of bridge items that require inspection for a successful Bridge Management System. Pontis is ‘bridge’ in Latin.

In 2010, the AASHTO Bridge Element Inspection Manual was developed to address improvements to the existing CoRe Element Guide. This reference manual was prepared by representatives from California, Idaho, Michigan, Montana, New York and FHWA to further enhance bridge management.

Significant changes from the existing CoRe Element Guide:

- All elements have four defined condition states having general descriptions (good, fair poor, and severe).
- Wearing surfaces have been separated from decks/slabs and protective coatings.
- Elements have been categorized as National Bridge Elements (NBEs) or Bridge Management Elements (BMEs), with provisions for custom agency developed elements.
- Multiple distress paths provide the ability to incorporate all defects within the overall element assessment.
- Smart Flags (Defect Flags) have been revised to identify the predominant distress.

4.3.2

Element Level Inspection Development

In developing a system for standardized data collection, the FHWA needed to look at the shortcomings of NBI (National Bridge Inventory) data. The problems with NBI data included:

- Each bridge is divided into only three major parts for condition assessment: deck, superstructure, substructure and culvert.
- The rating scale for these parts is 0-9 by severity of the deficiency, which does not indicate the extent of the deficiency.

- The component condition ratings are based on subjective interpretation by the inspectors.

A system was developed which included a standardized description of bridge elements at a greater level of detail. The FHWA created a task force to revise the standards and created a manual called "*Commonly Recognized (CoRe) Structural Elements*". The AASHTO Guide for CoRe Element Manual defined each element, the unit of measurement, definitions of a set of 3-5 standardized condition states, and feasible actions for each condition state. The CoRe Element Manual was accepted as an official AASHTO manual in May 1995. Some states developed their own CoRe Element Manual based on the AASHTO *Core Element Manual*. Approximately 40 states perform element level inspection.

In 2010, the limitations of the CoRe Element Manual were again addressed. These problems included:

- Inconsistent number of condition states and descriptions between element types
- Inconsistent condition state definitions between agencies
- Limited distress path language defined within the condition states

The National Bridge Element and Bridge Management Element system provides multiple distress paths for each defined condition state. This allows for deficiencies to be identified within each overall element assessment. The AASHTO *Guide Manual for Bridge Element Inspection* defines each element, description, unit of measurement or quantity calculation, set of four standardized condition states, feasibility actions, element commentary, and element definitions. The AASHTO *Guide Manual for Bridge Element Inspection, First Edition, 2011*, was first published as an official manual in February 2011.

4.3.3

Element Level Rating Terminology

The AASHTO *Guide Manual for Bridge Element Inspection, First Edition, 2011*. (see Figure 4.3.1) provides a description of structural elements that are commonly used in highway bridge construction and encountered on bridge safety inspections.

The following terms are used to describe bridge element-level inspection:

- National Bridge Elements (NBEs) represent the primary structural components of bridges necessary to determine the overall condition and safety of primary load carrying members. They provide a uniform basis for data collection.
- Bridge Management Elements (BMEs) represent a recommended set of condition assessment language that may be modified to suit the agency's needs. Examples of these elements include expansion joints and seals, approach slabs, wearing surfaces, protective coatings and smart flags.
- Agency developed elements are customized elements that can be sub-sets of defined NBEs, sub-sets of BMEs, or elements that are independent of the defined AASHTO elements. Agency developed elements are used in addition to the NBEs and BMEs.
- Condition states describe the severity of the deficiencies in AASHTO Bridge Elements. All elements have four defined condition states having general descriptions of good, fair, poor, and severe. Condition State 1 (good) and Condition State 4 (severe).
- Environments are used to classify the operating conditions and the deterioration of the structure, which does not change due to maintenance work or deficiencies. Depending on the agency, inspectors may or may not be responsible for determining the environment.
- Sub-elements or sub-sets are divisions of NBEs or BMEs that are created to provide flexibility to track variations in cost or performance characteristics.
- Smart Flags or Defect Flags are BMEs and used when a specific condition exists, which may be described in the National Bridge Element condition state definitions. They inherit the same units of measure as the NBE or BME to which they are assigned.
- Feasible actions, as provided in the AASHTO *Guide Manual for Bridge Element Inspection*, are general actions to address deficiencies. Feasible actions are often further defined by agencies for each condition state. Agency procedures vary and some inspectors create work recommendations for feasible actions. The inspector may not be required to record feasible actions.

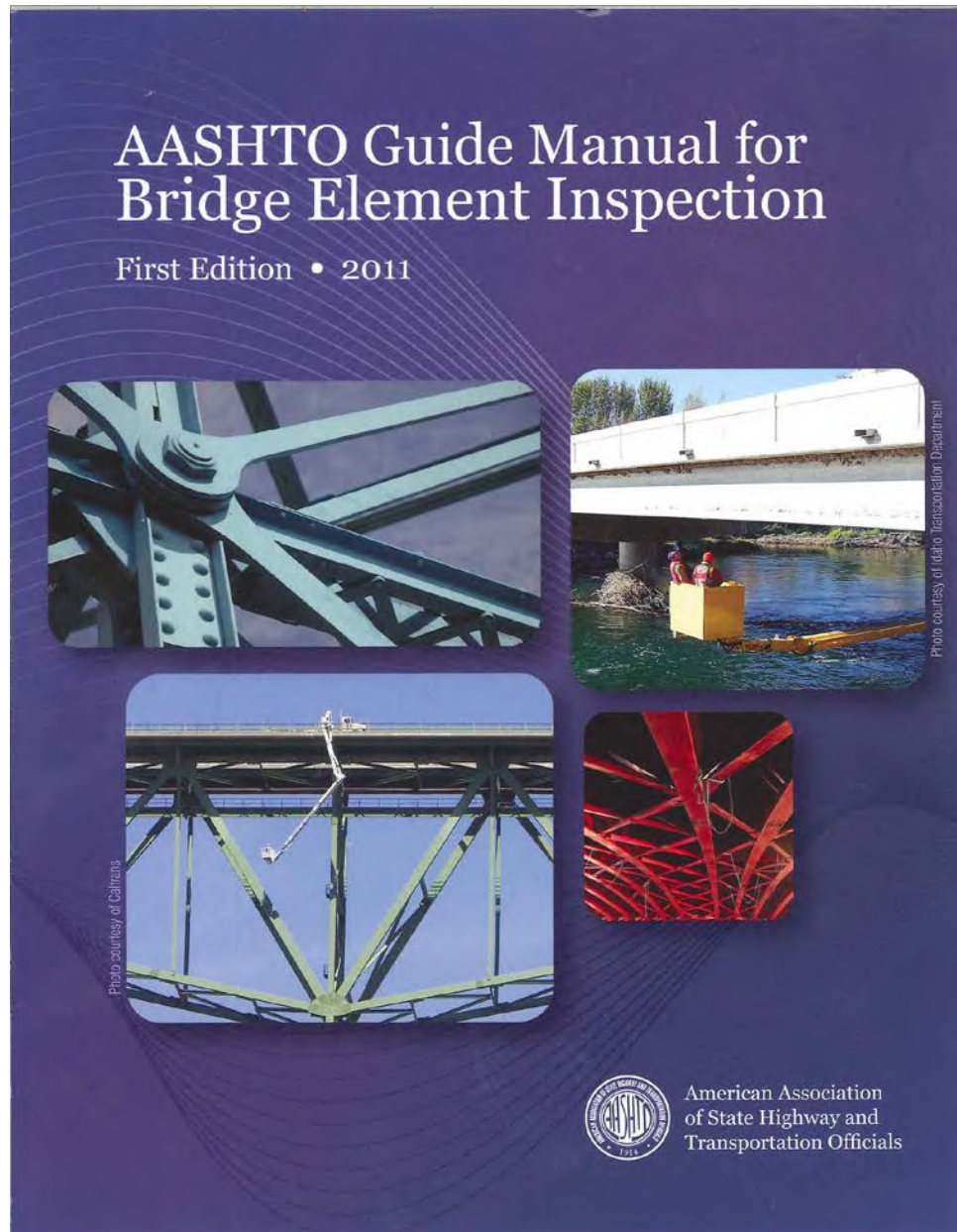


Figure 4.3.1 AASHTO Guide Manual for Bridge Element Inspection

4.3.4

Basic Requirements of National Bridge Elements

In the development of National Bridge Elements, it was important that the specification must be generic. Different agencies have varying maintenance practices, funding mechanisms, policy concerns and terminology. However, the physical components of bridges and deterioration processes are not unique. Agencies must be able to customize the generic standard to satisfy their own purposes without sacrificing the benefits of a common standard. Any changes to elements could introduce incompatibility between agencies. For this reason, agencies cannot change the number of condition states and the intent of the condition state language.

To avoid this from happening, the bridge element guide manual provides the ability of an agency to add custom agency developed elements or modify recommended

Bridge Management Elements. It is possible for future National Bridge Elements or Bridge Management Elements to be added. These elements must be permanent, have clear distinction and be defined as concisely as possible. The guidelines for developing National Bridge Elements include:

- Each element must be a primary load carrying element
- Each element must have a unique functional role.
- Distinguish elements that have significantly different maintenance requirements.
- Distinguish elements that are measured in different ways for costing or inspection.
- Distinguish elements whose conditions are described in different ways.
- Each element must be significant from the standpoint of maintenance cost or functionality. This is why, for example, secondary members are omitted from the list of National Bridge Elements. The level of detail in data collection would be too large relative to the effect of these elements on decision making.
- Deterioration behavior and maintenance alternatives for the element must be sufficiently understood. This is why, for example, composite materials such as fiber reinforced polymer are excluded from the list of National Bridge Elements.
- If an element is more significant than other elements, its behavior or condition description is complex, the element may be subdivided into smaller elements. An example of this type of element would be a pin and hanger assembly.
- A formal definition of each element must be developed to clarify thinking.

One primary use of definitions is to establish a useful inventory. In the field, each element must be clearly identified, measured and counted economically. It is also important to describe element attributes, such as size, material, condition and serviceability, quantitatively. The commonality aspect of National Bridge Elements depends on having definitions that are widely understood and are stable over time. One major factor contributing to definitions being widely understood is NHI's bridge inspection related training courses.

4.3.5

Bridge Element Identification

National Bridge Elements

AASHTO National Bridge Elements describe primary load carrying members, including:

- Girders
- Trusses
- Arches
- Cables
- Floorbeams
- Stringers

- Abutments
- Piers
- Pins and Hangers
- Culverts
- Bearings
- Railings
- Decks
- Slabs
- Gusset Plates
- Column/Piles
- Caps

See Figures 4.3.2 - 4.3.4 for a list of decks/slabs, superstructure, and substructure AASHTO National Bridge Elements.

Element	Units	Element Number (Decks)	Element Number (Slab)	Other
Reinforced Concrete Deck/Slab	AREA	12	38	
Prestressed/Reinforced Concrete Top Flange	AREA	15		
Steel Deck - Open Grid	AREA	28		
Steel Deck - Concrete Filled Grid	AREA	29		
Steel Deck - Corrugated/Orthotropic/Etc.	AREA	30		
Timber Deck/Slab	AREA	31	54	
Bridge Rail		Other		
Metal Bridge Railing	LENGTH			330
Reinforced Concrete Bridge Railing	LENGTH			331
Timber Bridge Railing	LENGTH			332
Other Bridge Railing	LENGTH			333
Masonry Bridge Railing	LENGTH			334

AREA = square feet (square meter)

LENGTH= feet (meters)

Figure 4.3.2 Decks/Slabs National Bridge Elements in the AASHTO *Guide Manual for Bridge Element Inspection*

Element	Units	Steel	Prestressed Concrete	Reinforced Concrete	Timber	Masonry	Other
Girder/Beam	LENGTH	107	109	110	111		
Closed Web/Box Girder	LENGTH	102	104	105			
Stringer	LENGTH	113	115	116	117		
Truss	LENGTH	120			135		
Arch	LENGTH	141	143	144	146	145	
Floor Beam	LENGTH	152	154	155	156		
Cable	EA	147, 148					
Gusset Plate	EA	162					
Pin and/or Pin and Hanger Assembly	EA	161					

LENGTH= feet (meters)
 EA = Each

Figure 4.3.3 Superstructure National Bridge Elements in the AASHTO *Guide Manual for Bridge Element Inspection*

Element	Units	Steel	Prestressed Concrete	Reinforced Concrete	Timber	Masonry	Other
Column/Pile Extension	EA	202	204	205	206		
Column Tower (Trestle)	EA	207			208		
Submerged Pile	EA	225	226	227	228		
Pier Wall	LENGTH			210	212	213	211
Abutment	LENGTH	219		215	216	217	218
Pier Cap	LENGTH	231	233	234	235		
Pile Cap/Footing	EA			220			
Culvert	LENGTH	240		241	242	244	243
Bearings							
Elastomeric Bearing	EA						310
Moveable Bearing (roller, sliding, etc.)	EA						311
Enclosed/Concealed Bearing	EA						312
Fixed Bearing	EA						313
Pot Bearing	EA						314
Disk Bearing	EA						315

LENGTH= feet (meters)
 EA = Each

Figure 4.3.4 Substructure National Bridge Elements in the AASHTO *Guide Manual for Bridge Element Inspection*

Bridge Management Elements

AASHTO Bridge Management Elements represent a recommended condition assessment language that can be modified to suit the agency's needs. The following types of elements are defined as Bridge Management Elements:

- Joints
- Approach Slabs
- Wearing Surfaces
- Protective Systems
- Smart Flags (Defect Flags)

See Figures 4.3.5 - 4.3.6 for a list of decks/slabs and wearing surfaces and protection systems AASHTO Bridge Management Elements.

Element	Units	Element Number
Joints		
Strip Seal Expansion Joint	LENGTH	300
Pourable Joint Seal	LENGTH	301
Compression Joint Seal	LENGTH	302
Assembly Joint/Seal (modular)	LENGTH	303
Open Expansion Joint	LENGTH	304
Assembly Joint w/o Seal	LENGTH	305
Approach Slabs		
P/S Concrete Approach Slab	AREA	320
Reinforced Concrete Approach Slab	AREA	321

AREA = square feet (square meter)

LENGTH= feet (meters)

EA = Each

Figure 4.3.5 Decks/Slabs Bridge Management Elements in the AASHTO *Guide Manual for Bridge Element Inspection*

Element	Units	Element Number
Protective Systems		
Wearing Surfaces	AREA	510
Steel Protective Coating	AREA	515
Deck/Slab Protection Systems	AREA	520
Concrete Protective Coating	AREA	521

AREA = square feet (square meter)

Figure 4.3.6 Wearing Surfaces and Protective Systems in the AASHTO *Guide Manual for Bridge Element Inspection*

Defect Flags

Defect Flags are part of the Bridge Management Elements and are used to identify the predominant defect for that condition state. The severity of the deficiency is captured by coding the appropriate Defect Flag condition state. The NBI translator uses AASHTO element-level data that includes defect flag data to determine NBI component condition ratings.

Defect Flags inherit the units of the parent NBE or BME.

Steel Cracking/Fatigue:	This flag shall be used with steel elements to identify the predominant defect in a given condition state that is not corrosion.
Pack Rust:	This flag shall be used in conjunction with steel elements connection defects (including shapes in contact in built-up members) of steel bridges that are already showing signs of rust packing between plates.
Concrete Cracking:	This flag shall be used with concrete elements to identify the predominate defect in a given condition state that is not spalling or delaminations.
Concrete Efflorescence:	This flag shall be used with concrete elements to identify the predominate defect in a given condition state that is not spalling or delaminations.
Settlement:	This flag shall be used with all substructure and culvert elements to identify the predominate defect in a given condition state that is not material deterioration. The use of the flag is to identify the severity of the settlement.
Scour:	This flag shall be used with all substructure and culvert elements to identify the predominate defect in a given condition state that is not material deterioration. The use of the flag is to identify the severity of the scour.
Superstructure Traffic Impact:	This flag shall identify all traffic collisions with the superstructure. Application of the flag is in relation to the impact on the structures capacity to carry load.
Steel Section Loss:	This flag shall be used with steel elements to identify the predominate defect in a given condition state that is not corrosion. Setting this flag will identify the severity of section loss.
Steel Out-of-plane Compression Members:	This flag shall be used with steel truss or arch elements. The use of the flag shall denote any member that is not in plane with the panel (buckling). It shall be used to identify the predominate defect in a given condition state that is not material deterioration.
Deck Traffic Impact:	This flag shall identify all traffic collisions with the deck. Application of the flag is in relation to the impact on the structures capacity to carry load.

Substructure Traffic Impact:	This flag shall identify all traffic collisions with the substructure. Application of the flag is in relation to the impact on the structures capacity to carry load.
Barrel Distortion:	This flag is to identify the severity of the culvert barrel distortion. Its use shall be with culverts only. This flag shall describe predominate culvert deterioration that is not attributed to material deterioration.

Agency Developed Elements

Agencies may develop sub-elements that use the same condition state definitions as their associated NBE or BME elements. This allows for more detailed element descriptions. They are a subset of the NBE or BME and allow a more detailed classification. They are often created to distinguish a different size, location or exposure.

- Fascia girders and interior girders can be examples of Sub-Elements.
- The ends of girders can be examples of Sub-Elements.

Agency developed elements fall into three main categories: subsets of NBEs, BMEs, or elements that are independent of defined elements. Agency Developed Element guidelines are listed below:

Agency Defined Subsets of NBEs

For agency defined sub-sets of National Bridge Elements, the agency must be able to combine the sub-elements back together to form the original NBE element for NBI submission with the original condition state and element definition language.

Agency Defined Subsets of BMEs

For agency defined sub-sets of Bridge Management Elements, the agency is not required to combine the elements to form the original Bridge Management Elements since BMEs are not required for NBI submission. However, custom elements of this type must retain the original number of condition states using a good, fair, poor, severe description.

Independent Agency Developed Elements

For Agency Defined Elements that are not sub-sets of National Bridge Elements or Bridge Management Elements, the only requirement is the standardized number of condition states (four). These elements may include inventory items or specific aspects of the structure. Independent Agency Defined Elements may or may not include feasible actions, deficiency, or official condition state language.

Examples of potential independent agency developed elements include approach guardrail, approach guardrail ends, seismic retrofit components, tunnels, condition of drainage components or lighting fixtures, or ancillary items such as overhead signing structures.

4.3.6

Condition States

The scale of good-fair-poor-severe is not acceptable because these terms do not have precise definitions that can be observed in the field. It was decided to measure bridge condition on a single scale that reflects common processes for deterioration and the effect on serviceability. The general pattern for a Bridge Element having four condition status is as follows:

1. Good – No deterioration to minor deterioration
2. Fair – Minor to Moderate deterioration
3. Poor – Moderate to Severe deterioration
4. Severe – Beyond the limits established in condition state 3 and/or warrants a structural review to determine strength or serviceability of the element or bridge

Each of these levels of deterioration is called a condition state. The condition state methodology provides two types of information about a bridge element's deterioration:

- Severity – characterized by precise definition of each condition state
- Extent – the distribution of the total element quantity among condition states

The severity is important for selection of a feasible and cost effective preservation treatment, and extent is important for cost estimation.

Assignment of quantities to condition states is determined from element definitions and element commentary for National Bridge Elements. Condition state definitions are guidelines to the bridge inspector for categorization of the severity of the deficiency. Element commentary represents additional considerations for the inspector during the collection of data. From this information, the inspector can complete the element level evaluation. Additionally, element level Smart Flags (Defect Flags) are used to describe a condition which is not included in the National Bridge Element or Bridge Management Element condition state language.

4.3.7

Feasible Actions

Feasible actions are those that an agency may take to remove the defect. They represent a set of responses that may be taken for an element based upon quantities within a given condition state. They also represent general guidance on agency preservation strategies and can be customized by each agency for each element and condition state.

A summary of feasible actions and associated condition states is given below. Depending on the element, some feasible actions/conditions states may not be available. Other feasible actions, such as "Do Nothing", are available for all elements and condition states. "Do Nothing" can be used for all the elements in condition states since the possibility of nothing that needs to be done due to the condition of the element being good or to be used if the condition of the bridge is

so severe, the bridge is closed and or there is a feasible action already taking place.

Feasible Action	Condition State			
	1	2	3	4
Do Nothing	●	●	●	●
Protect	●	●	●	●
Preserve (for other culverts and other railings)	●	●		
Repair		●	●	●
Rehab			●	●
Reset (for bearings only)			●	●
Replace			●	●

4.3.8

Environments

Element can exist in one of four environments, which describe different weather or operating conditions. The environments are important for deterioration models and prediction of future conditions. The four environments are defined in general terms as follows:

1. Benign – No environmental or operational conditions affecting deterioration
2. Low – Environmental or operational conditions create no adverse impacts, or are mitigated by past non-maintenance actions or highly effective protective systems
3. Moderate – Typical level of environmental or operational conditions influence on deterioration
4. Severe – Environmental or operational conditions factors contribute to more rapid deterioration. Protective systems are not in place or are ineffective

Environment policies are used for element level inspection and set by individual state agencies.

4.3.9

The Role of Element Level Data in Bridge Management Systems

An immediate application of Bridge Elements is the collection and analysis of performance data. It is essential that original data collection be as objective and repeatable as possible. This raw, objective data must be stored so that the analysis may be updated or improved at a later time. Bridge Elements must be usable to support management decision making. The large volume of raw data collected must be transformed into useful information. For this reason, the development of bridge Bridge Elements was heavily influenced by the parallel development of Pontis software and previous CoRe elements.

Condition state data provides quantitative data about the physical condition and performance of bridge elements. This data is also, the effects of treatment actions can be tracked over time. Element level data is an essential part of the following BMS functions. Element level inspections can track the effectiveness of action over time by showing the various condition states and how they may change over time after the bridge element is either repaired, replaced, or nothing would be done. Potential applications for agencies includes:

- Identification of bridge needs (replacement and preservation)
- Development and testing of new maintenance techniques
- Treatment selection policies
- Project priority setting and programming
- Budgeting
- Funding allocation
- Long-range planning

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Abbreviations for Field Inspection Notes

Abut. = Abutment	Hvy. = Heavy
Adj. = Adjacent	Int. = Interior
B. = Bent	Lac. = Lacing
Btw. = Between	Lat. = Lateral
Bot. = Bottom	Lat. Br. = Lateral Brace
B.S. = Both Sides	Lgth. = Length
[= Channel (Steel Shape)	Low. = Lower
cm = Centimeter	Lt. = Light
Col. = Column	M = Meters
Conc. = Concrete	Med. = Medium
Cond. = Condition	Mid. = Middle
Conn. = Connection	N = North
Cr. = Crack	No Vis. Def. = No Visible Defects
Delam. = Delamination, Delaminated	N.S. = Near Side
Deter. = Deterioration	P = Pier
Diag. = Diagonal	Pl. = Plate
Diam. = Diameter	S = South
Diaph. = Diaphragm	S.I.P. = Stay-in-Place Forms
D.S. = Downstream	SF = Square Feet
E = East	Stiff. = Stiffener
Eff. = Efflorescence	Str. = Stringer
Elev. = Elevation	T. Welds = Tack Welds
Exp. = Expansion	Typ. = Typical
F.B. = Floorbeam	U = Upper
F.L. = Full Length	U.S. = Upstream
Flg. = Flange	Vert. = Vertical
F.S. = Far Side	Vis. = Visible
Ft. = Feet	Vis. S. = Visible Signs
Gus. = Gusset	W = West
H.L. = Hairline	W = Wide Flange (Steel Shape)
Horz. = Horizontal	L = Angle (Steel Shape)

Topic 4.4 Record Keeping and Documentation

4.4.1

Introduction

Bridge owners maintain a complete, accurate, and current record of each bridge under their jurisdiction. Such information relating directly to the inspection, design, performance and maintenance of the bridge is vital to the effective management of a population of bridges. Additionally, this information provides a record that may be important for repair, rehabilitation, or replacement of their assets.

The first section in this topic covers the critical components of the bridge record, while the remaining sections provide the inspector with guidance on how to thoroughly organize inspection data and produce an accurate and effective inspection report.

4.4.2

Bridge Records

Bridge records, or files, are used to maintain detailed, cumulative and up-to-date information on each structure. A thorough study of the available historical information can be extremely valuable in identifying possible critical areas of structural or hydraulic components and features.

The contents of any particular bridge file may vary depending upon the size and age of the structure, the functional classification of the road carried by the structure, and the informational needs of the agencies responsible for inspection and maintenance. The bridge file is not only a resource to the bridge owner, but also a resource to the inspector. The inspector will gain valuable insight into the bridge by being familiarized with it prior to the inspection. It is recommended that the following types of information be assembled when possible.

According to the *AASHTO Manual for Bridge Evaluation*, the bridge record includes the following information:

- Plans, including construction plans, shop and working drawings, and “as-built” drawings
- Specifications
- Correspondence
- Photographs
- Materials and tests, including material certification, material test data, and load test data
- Maintenance and repair history
- Coating history
- Accident records
- Posting
- Permit loads
- Flood and scour data
- Traffic data
- Inspection history
- Inspection requirements

- Structure Inventory and Appraisal sheets
- Inventories and inspections
- Rating records

Plans

Construction, “as-built,” or shop and working plans are included in a bridge record. If plans are not available, determine the following types of construction information: date built; type of structure, including size, shape, and material; design capacity; and design service life. Hydraulic data is also assembled where available, including structure profile gradeline, elevation of inverts or footings, stream channel and water surface during normal and high flows, design storm frequency, drainage area, design discharge, date of design policy, flow conditions, limits of flood plain, type of energy dissipaters (if present), cut-off wall depth, channel alignment, and channel protection.

Specifications

The bridge record includes a complete copy of the technical specifications used to design and build the bridge. When a general specification was used, only the special provisions are included in the file. The edition and date of the general specifications are noted in the bridge record.

Correspondence

The bridge record includes any applicable letters, memorandums, and notices of project completion, construction diaries, telephone logs, and any other information directly concerning the bridge in chronological order.

Photographs

Photographs are used to supplement the inspection notes and sketches. A minimum of two photographs are included in the bridge record: a topside view of the bridge roadway and at least one elevation view of the bridge. Photographs showing major deficiencies or other features, such as utility attachments or channel alignment, also are included. Photographs that show load posting signs are also provided, if applicable.

Photo Log

Keep a photo log during the inspection. The photo log includes the date, photo number, and description of each photograph. It is best to be very specific when describing the photos (see Figure 4.4.1). Descriptions include both the location of the member and a brief description of any deficiencies.

PHOTO LOG FORM

INSPECTORS GRL (WB) PHOTO LOG NO. _____
 BRIDGE NO. MP-056-0064-B00293 DATE: 7.9.11 (Photos 1-11)
 BRIDGE NAME 7th Street to 13th Street Unit 7.10.11

PHOTO #	LOCATION	DESCRIPTION
1	Pier 46WB from parking level	General View Sta Abd at end of Ramp (left) beg. at Pier 47WB
2	Pier 49WB	Crack at end weld at Girder Conn (S9-Span 48WB)
3-7	Pier 49W Span 49W	Gen View of end weld crack w/ close-ups
8	North side Pier 50WB	Gen View of Fascia Girder (S19) w/ full length long stiffeners
9	Pier 55WB	torc strip seal over cross girder and pin & hanger assembly
10	Pier 55WB (north side)	Top View of Deck Looking Bk Stations
11	Pier 55WB	Pin & Hanger Assembly w/ Bot support (Looking North)
12	Span 48WB Girder S5	2nd Diaph (Cross Bracing) Top Conn & end weld cracking and toe weld cracking on Girder web in Flg (Previously reported - on change) opposite end of how similar condition

Figure 4.4.1 Sample Photo Log

Materials and Tests

Certificates for the type, grade, and quality of materials used in construction of the bridge are included in the bridge record. Examples include steel mill certificates, concrete delivery slips, and any other manufacturers' certificates. The certificates are retained in accordance with bridge owner policy and statute of limitations.

Reports for any non-destructive or laboratory testing either during or after construction are included. If any field load testing is performed, provide the reports in the bridge record.

Maintenance and Repair History

Information about repairs and rehabilitation activities are included in the bridge record. This chronological record includes details such as the date, project description, contractor, cost, contract number and any other related data. The types and amount of repairs performed at a bridge or culvert site can be extremely useful. For example, frequent roadway patching due to recurring settlement over a culvert or approach roadway for a bridge may indicate serious problems that are not readily apparent through a visual inspection of the structure.

Coating History

This information in the bridge record documents the surface protective coatings used, including surface preparation, application method, dry film paint thickness, types of paint, concrete and timber sealants, and other protective membranes.

Accident Records

Include details of accidents or damage to the bridge in the bridge record (see Figure 4.4.2). This information includes the date of the occurrence, description of the accident, member damage and repairs, and any investigative reports.



Figure 4.4.2 Accident Involving Construction Equipment and a Bridge

Posting

Each bridge record includes load capacity calculations and any required posting arising from the load ratings. The summary of posting actions includes the date of posting and a description of the signing used (see Figure 4.4.3).



Figure 4.4.3 Posted Bridge

Permit Loads

A record of the most significant single-trip permit loads using the bridge are included in the bridge record. This information is to include any applicable documentation and calculations.

Flood and Scour Data

A chronological history of major flooding events are included for bridges over water (see Figure 4.4.4). This history includes the high water marks at the bridge site, scour evaluation, scour history, and any plan of action.



Figure 4.4.4 Flood Event

Traffic Data

When available, the bridge record contains a history of the variations in Average Daily Traffic (ADT) and Average Daily Truck Traffic (ADTT) including the frequency and types of vehicles using the bridge. ADT and ADTT are important factors in determining fatigue life and are monitored for each bridge and each traffic lane on the bridge. If available, weights of the vehicles using the bridge are also included in the bridge record.

Inspection History

Reports from previous inspections can be particularly useful in identifying specific locations that require special attention during an inspection. Information from earlier inspections can be compared against current conditions to estimate rates of deterioration and to help judge the seriousness of the problems detected and the anticipated remaining life of the structure.

This chronological record of inspections performed on the bridge includes the date and type of inspection. The initial inspection report is included in the bridge record. Earthquake data, fracture critical member information, deck evaluations, and corrosion studies are also included when available.

Inspection Requirements Inspections are planned and prepared for by taking into account needed access, inspection equipment, structural details, inspection methods, and the required qualifications of inspection personnel. In addition, the National Bridge Inspection Standards require that written inspection procedures for specific types of more complex inspections (fracture critical, underwater, and complex bridges) be developed to address those items that need to be communicated to an inspection team leader to ensure a successful bridge inspection. Section 4 of the AASHTO *MBE* has general considerations regarding inspection plans. An owner may have general overall inspection procedures in their bridge inspection manual that address common aspects of these more complex inspections, however, each bridge will have written inspection procedures specific to each bridge which address items unique to each bridge. The following items are to be addressed for each of these types of bridge inspections, either in the bridge specific inspection procedures, or by referring to general inspection procedures (typically in an agency's bridge inspection manual):

- Identify each of the critical members to be inspected (fracture critical elements, past repairs, underwater elements, complex features, fatigue prone details, scour countermeasures, etc.) on plan sheets, drawings or sketches
- Identify special access needs or equipment necessary to gain the access required to inspect the features (under bridge inspection trucks, man lifts, traveler system, climbing, etc.)
- Describe the inspection method(s) and frequency to be used for the elements. For example, “Visually inspect all identified FCMs at arm’s length for cracks, deterioration, missing bolts, loose connections, broken welds... using PT to verify the existence of suspected cracks.”
- Address required proximity to details, such as “arm’s length”
- Identify special qualifications required of inspection personnel by the program manager, if any (successfully passed fracture critical course, certified electrician for movable bridge electrical components, qualified bridge inspection diver, etc., may be possible qualifications)

Other items that may be addressed depending on each unique situation might include:

- Special contacting procedures prior to inspection (Coast Guard, security, operations personnel, etc.)
- Safety concerns (snakes, bats, etc.)
- Best time of year to inspect the bridge (lake draw down, canal dry time, snow, ice, bird nesting seasons, etc.)
- Anything else the program manager wants the inspection team leader to be aware of in preparation for the inspection

Any special requirements to ensure inspector and public safety, including a traffic management plan, are also included.

Structure Inventory and Appraisal Sheets A chronological record of SI&A forms used by the bridge owner is included in the bridge record. Refer to Topic 4.1 for a complete description of SI&A sample form.

Inventories and Inspections Inspection reports are included as part of the bridge record. This information includes the results of all inventories and bridge inspections and can include construction or repair activities.

Bridge Inspection Forms

Many bridge owners have standard inspection forms. These forms are used for each bridge in their system and give the inspector a checklist of items that are to be reviewed. Another benefit of standardized forms is that it organizes bridge reports into a consistent format (see Figures 4.4.5 and 4.4.24 that are located at the end of this topic).

Rating Records

A complete record of the determination of the bridge's load-carrying capacity is included in the bridge record (see Figure 4.4.6). This information will include the design load to indicate the live load the bridge was designed for, the analysis methods used to determine the inventory and operating ratings, and the inventory and operating ratings for the bridge. The capacity calculations will be signed and dated by the individual who determined them, together with any assumptions used.

Nebraska Department of Roads - Bridge Division Load Rating Summary Sheet			
State Bridge Number	C008500335	Analyst	
County Bridge Number	T4N R4W SEC 8 L	Analysis Date	3/8/2007
Structure Type	Concrete continuous -- Slab	Year Built	1998
Highway System	Not on National Highway System	Year Reconstructed	
		Design Load	HS20
NBI Rating Factor Summary (HS or HL93):			
Inventory Capacity	1.11	Operating Capacity	1.85
Legal Truck Summary:			
Type 3 (Tons)	61	Type 3S2 (Tons)	67
		Type 3-3 (Tons)	101
Recommended Posting Summary:			
Type 3 (Tons/NA)		Type 3S2 (Tons/NA)	
		Type 3-3 (Tons/NA)	
Posting is required for capacities less than 25T, 37T, and 43T respectively. Gross Posting should be avoided.			
Permit Load Summary:			
Type 3 (Tons)	79	Type 3S2 (Tons)	87
		Type 3-3 (Tons)	132
For permitting purposes only, capacity based on a single lane distribution factor with no impact. No other vehicles are to be allowed on the bridge, crawl speeds less than 5 mph, and no gear shifting or braking, are to be strictly observed			
Rating Method: <input type="checkbox"/> ASR <input checked="" type="checkbox"/> LFR <input type="checkbox"/> LRFR <input type="checkbox"/> Other			
Rating Information Provided: <input checked="" type="checkbox"/> Plans <input type="checkbox"/> Field Measurements <input type="checkbox"/> Testing <input type="checkbox"/> No Information Exists			
Depth & Type of Overlay: 1 in. <input checked="" type="checkbox"/> Concrete <input type="checkbox"/> Gravel <input type="checkbox"/> Asphalt <input type="checkbox"/> Other			
Condition Rating:			
Deck:	9	Superstructure:	9
		Substructure:	9
		Pile:	9
		Scour:	8
Load Rating Evaluation Summary: I = Investigated C = Controls (HS or HL93)			
<input checked="" type="checkbox"/> <input checked="" type="checkbox"/> + M of Interior Girder / Beam	<input type="checkbox"/> <input type="checkbox"/> Truss Members		
<input checked="" type="checkbox"/> <input type="checkbox"/> + M of Exterior Girder / Beam	<input type="checkbox"/> <input type="checkbox"/> Floor Beams		
<input checked="" type="checkbox"/> <input type="checkbox"/> - M of Interior Girder / Beam	<input type="checkbox"/> <input type="checkbox"/> Stringers		
<input checked="" type="checkbox"/> <input type="checkbox"/> - M of Exterior Girder / Beam	<input type="checkbox"/> <input type="checkbox"/> Pins		
<input type="checkbox"/> <input type="checkbox"/> Shear At/Near Reactions	<input type="checkbox"/> <input type="checkbox"/> Hangers		
<input type="checkbox"/> <input type="checkbox"/> Deck Overhang	<input type="checkbox"/> <input type="checkbox"/> Substructure Elements		
<input type="checkbox"/> <input type="checkbox"/> Deck Between Girders	<input type="checkbox"/> <input type="checkbox"/> Sidewalks/Medians w/o Traffic Barriers		
<input type="checkbox"/> <input type="checkbox"/> Fatigue Prone Details	<input type="checkbox"/> <input type="checkbox"/> Scour		
Additional Comments (Include any section loss, location of section loss, assumptions, and hand calculation references used in this analysis)			
(Seal & Date)			
The recommended Rating and Posting for this structure are based on a theoretical analysis of the structural elements involved, and on a limited amount of information concerning their condition. These weight limits are intended only as a general guideline and may be varied accordingly by the officials responsible for this structure after an investigation of the structural condition, reaction to vehicular loads and any other items where judgement is required to establish a proper weight limit.			
DR Form 464, Jan 07			

Figure 4.4.6 Example Load Rating Summary Sheet

Post or restrict the bridge in accordance with the AASHTO *Manual for Bridge Evaluation* or in accordance with State law, when the maximum unrestricted legal loads or State routine permit loads exceed that allowed under the operating rating or equivalent rating factor.

4.4.3

Methods of Inspection Documentation

Traditional

Note all signs of distress and deterioration with sufficient precision so that future inspectors can readily make a comparison of conditions. The most commonly used method for record keeping is pencil and paper. The inspector writes findings on forms, sketches, and notebooks (see Figure 4.4.7). This method is extremely flexible in that the inspector can draw whatever configurations are necessary to best describe and document deficiencies.



Figure 4.4.7 Inspector Taking Notes

Electronic Data Collection

Another method of record keeping is electronic data collection (see Figure 4.4.8). This technology provides a significant advantage in a number of areas. With all the bridge data available at the site, the inspector can retrieve and edit previous records and save them as current inspection data. This not only saves time but eliminates the need for reentering data. Also, it eliminates errors that can occur when transferring the inspector's field notes to the computer back at the office. Electronic data collection provides a logical and systematic sequence of inspection, ensuring that no bridge elements are overlooked. It also allows the inspector to compare the current deficiencies with previous reports and note if any deterioration has gotten worse.



Figure 4.4.8 Electronic Data Collection

4.4.4

Inspection Report Documentation

While the inspection of small bridges usually only requires the use of the standard inspection form, the inspection of large or complex bridges requires the use of an inspection file, in addition to any standard inspection forms. The inspection file contains:

- Standard nomenclature and abbreviations for the elements of members and the components made up of these members
- Sketches of elements or members showing typical and deteriorated conditions (some of these can be pre-made to allow more expediency during the inspection)
- A standard notation system for indicating the condition of the elements or members
- A log or index for photographs
- Brief narrative descriptions of general and component conditions

When the above, detailed file format is selected for recording bridge inspection results, the information is to be recorded systematically. However, many bridge owners differ significantly in their required format. Most of the above information, if not provided on the inspection report, is available in the bridge record.

Element Identification

Identify the elements by the type of material, construction method, and the function that each element or member performs.

Some examples of elements or members and their abbreviations:

- Multi-beam (B1 – B6)
- Deck or slab
- Stringer (S1 – S4)
- Floorbeam (FB0 – FB15)

- Girder (G1, G2)
- Truss chord (U0U1 – U.S.)
- Truss diagonal (U0L2 – D.S.)
- Secondary bracing (Top Lat. Br. U0 U.S. to U1 D.S.)
- Arch
- Spandrel column (Col. 1 – Col. 14 – U.S.)
- Spandrel wall (U.S., D.S. or N, S, E, W)
- Abutment (Abut. 1, Abut. 2)
- Pier (P1 – P4)

Verify that element descriptions or abbreviations are consistent with bridge owner nomenclature.

Structure Site Orientation

Structure site orientation is normally established according to highway direction of inventory, mile markers, segments, or stationing. It is important that the orientation of each bridge be clearly established. The following are some examples:

- I79, Milepost 155.28 NB
- SR0019 Segment 05010
- Union Township, Alpha Drive, Station 109+05

Bridge Member Orientation

When describing bridge members, it is important to clearly identify the specific element or member that has the deficiency. The following are some examples to orient bridge members:

- Substructure units (e.g., Abutment 1 and Pier 3) (see Figure 4.4.9).
- Floorbeam ends are identified by left/right looking in the direction of inventory or north/south or east/west designations.
- Sides of members can be identified by direction (e.g., “south side of Floorbeam 2” or “northeast elevation of Beam 4”).
- Span numbers and bay numbers to identify general areas on the bridge (see Figure 4.4.9).
- Individual beams or stringers left to right, looking in the direction of inventory (see Figure 4.4.10).
- Upstream or downstream designations can be assigned to structures over waterways (e.g., “upstream truss”, “downstream girder”, or “upstream arch”) (see Figure 4.4.11).
- For truss elements, identify the member with joint designations and specify if it is an upstream/downstream or north/south truss (see Figure 4.4.12). Number floorbeams in accordance with the panel point numbers.

If the orientation used during the inspection differs in any way with that used in existing documents, clearly state these differences in the inspection notes.

ELEVATION VIEW

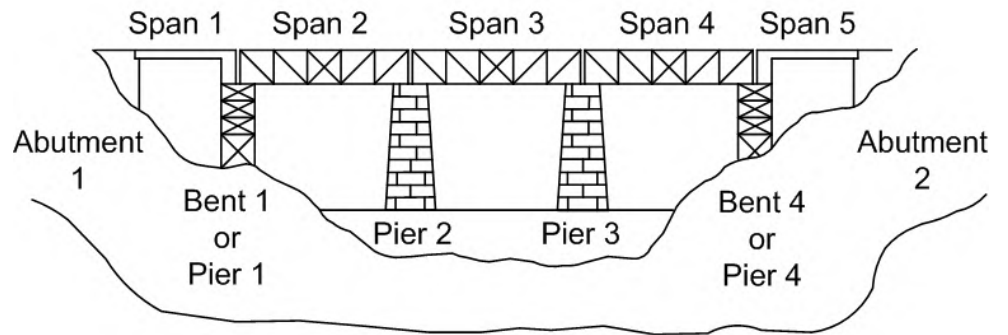


Figure 4.4.9 Sample Span Numbering Scheme

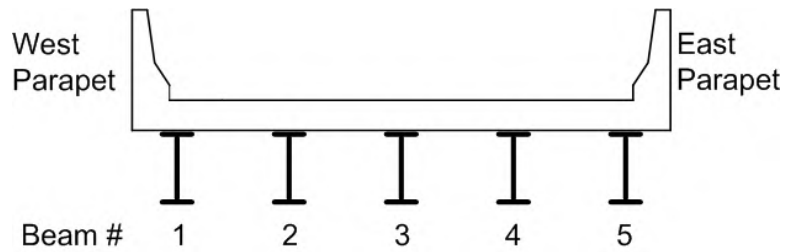
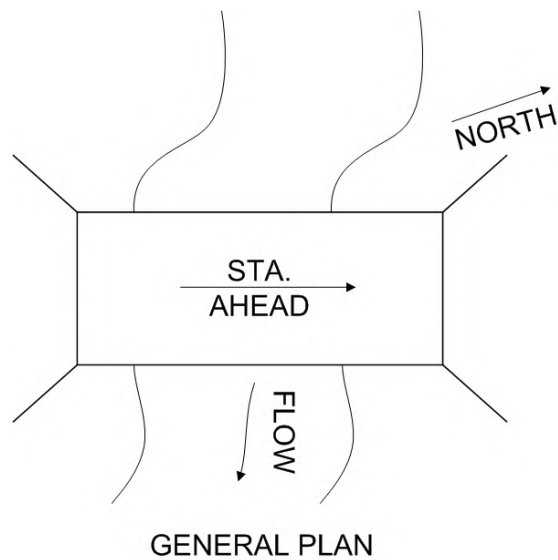


Figure 4.4.10 Sample Typical Section Numbering Scheme



GENERAL PLAN

Figure 4.4.11 Sample Structure Orientation Sketch

ELEVATION VIEW

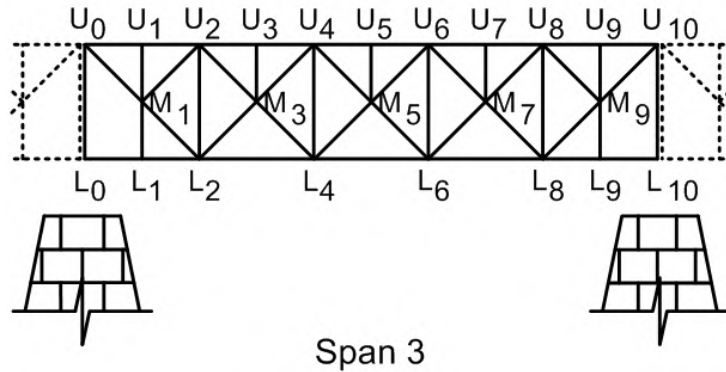


Figure 4.4.12 Sample Truss Numbering Scheme

Element Dimensions

Document sufficient dimensions to establish the size or cross section and other pertinent dimensions of elements. These include:

- Deck elements: length, width, and thickness
- Superstructure elements (beam, girder, floorbeam, stringer, and truss member): length, depth, width, flanges, and webs (see Figures 4.4.13 and 4.4.14)
- Substructure elements (abutment, columns and caps): width and depth (for rectangular shapes), diameter (for round columns), length, spacing, and pile batter and spacing (for pile bents)

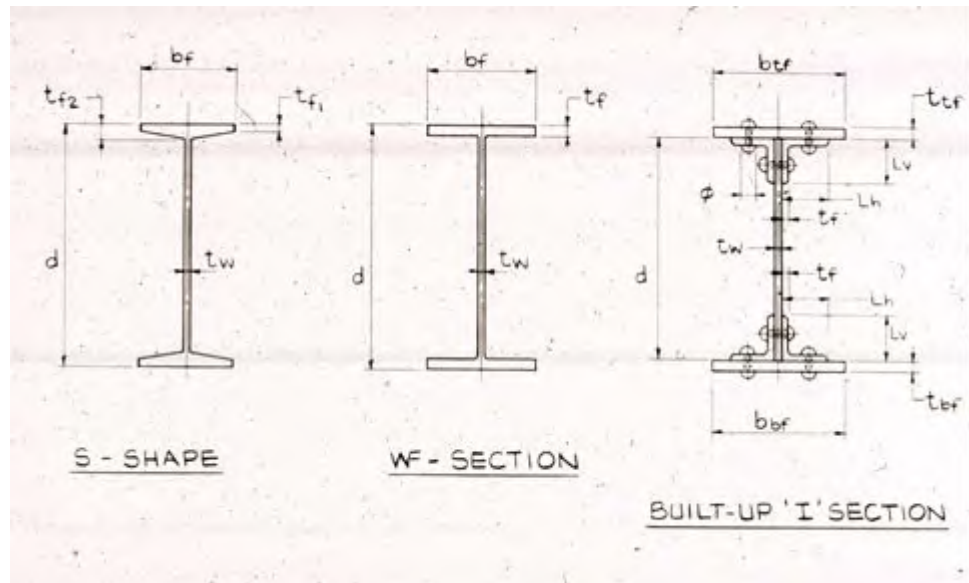


Figure 4.4.13 Steel Superstructure Dimensions

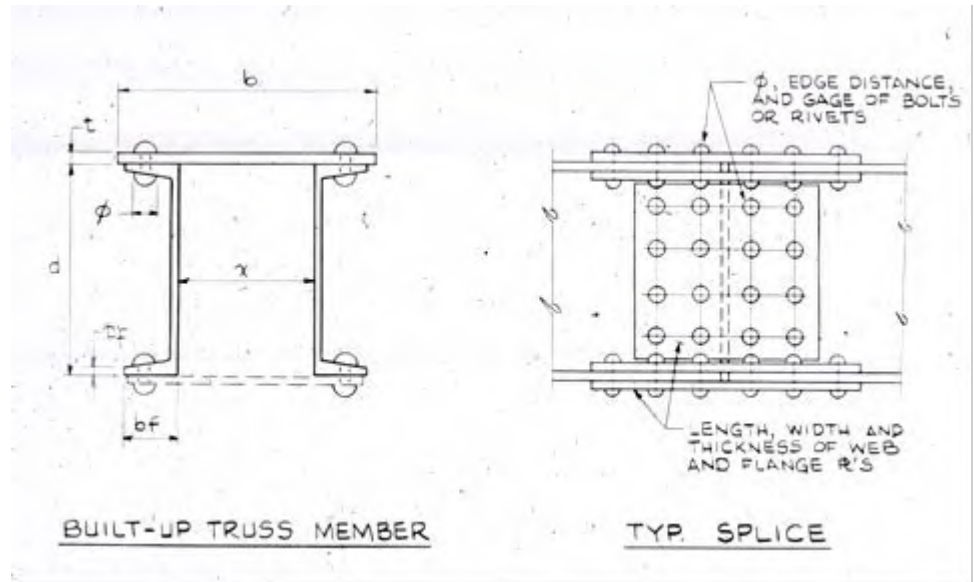


Figure 4.4.14 Truss Member and Field Splice Dimensions

Exact member dimensions are required to determine section properties used to calculate a load-rating analysis.

Inspection Notes and Sketches

In most cases, it will be possible to insert reproductions of portions of the plans in the inspection notes. However, in some instances, sketches will have to be drawn. The inspector may be able to pre-draw the sketches in the office and fill them out in the field (see Figures 4.4.15 through 4.4.17).

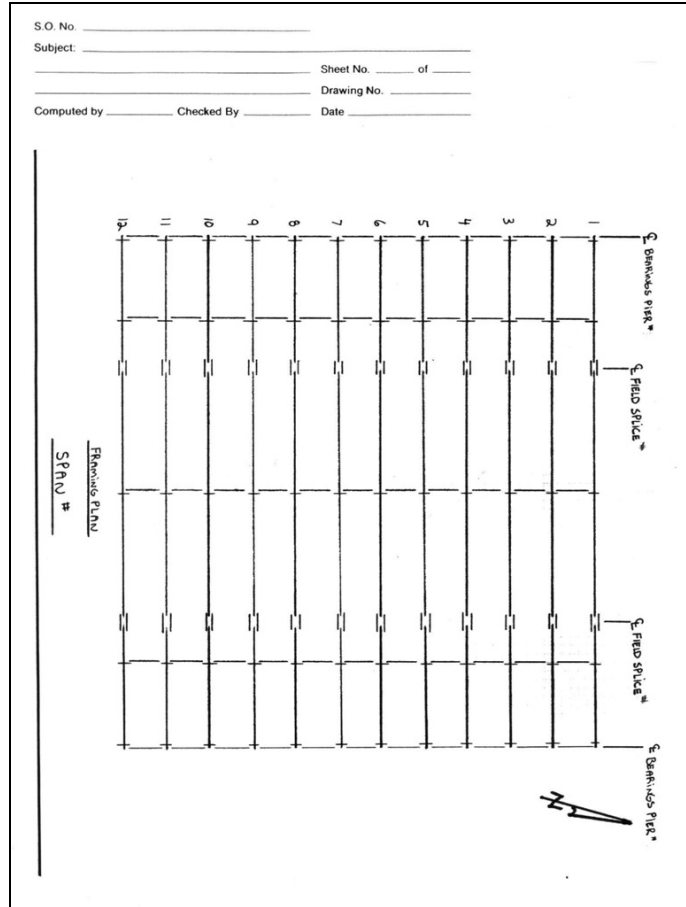


Figure 4.4.15 Framing Plan

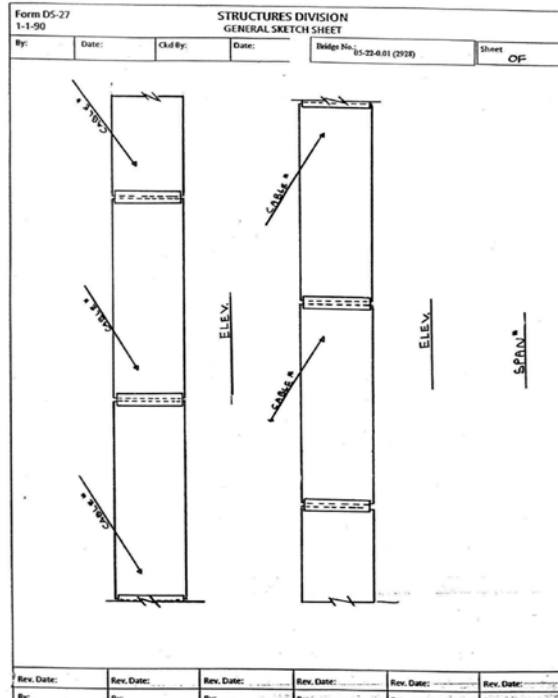


Figure 4.4.16 Girder Elevation

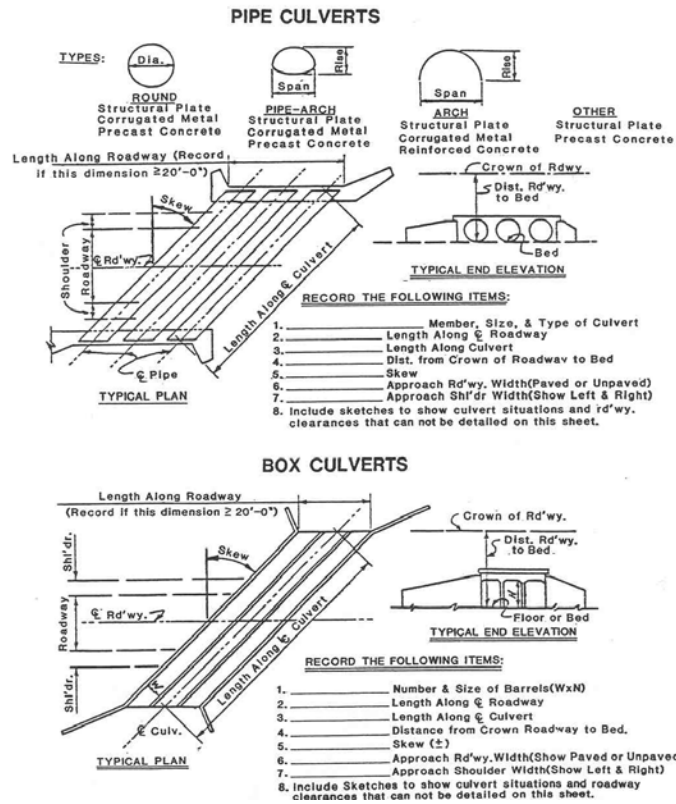


Figure 4.4.17 Typical Prepared Culvert Sketches

The first sketch in the field inspection notes normally portrays the general layout of the bridge and site information, illustrating the structure plan and elevation data (see Figures 4.4.18 and 4.4.19). The immediate area, the stream or terrain obstacle layout, major utilities, and any other pertinent details are also included.

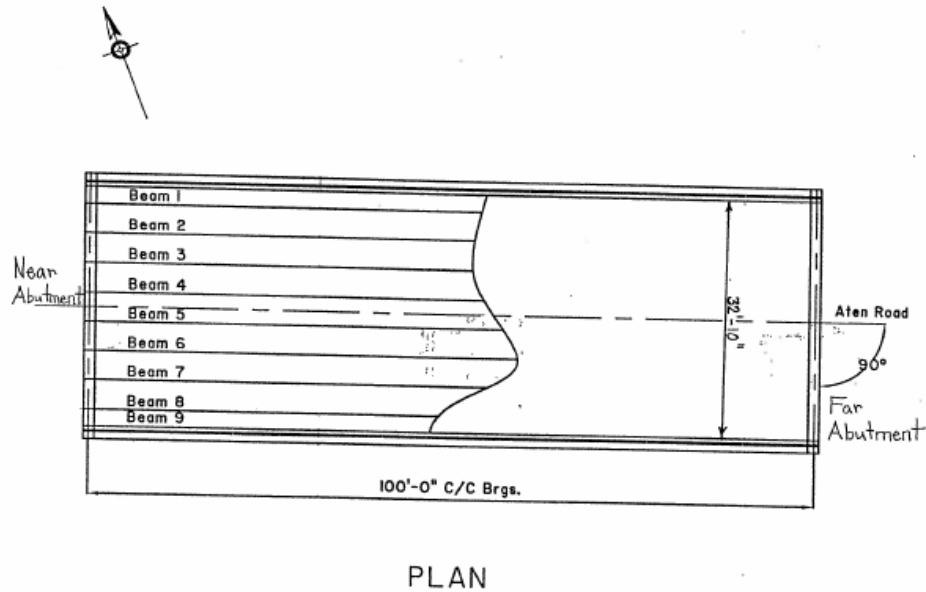


Figure 4.4.18 Sample General Plan Sketch

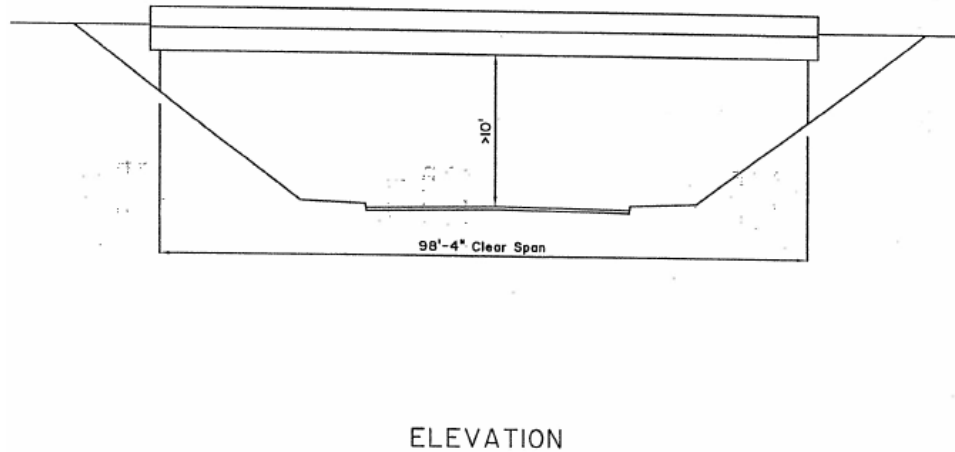


Figure 4.4.19 Sample General Elevation Sketch

Deficiency Identification Identify material deficiencies. as presented in Topic 6.1 – Timber, Topic 6.2 – Concrete, Topic 6.3 – Steel, Topic 6.5 – Masonry.

The exact location, severity and extent of deficiencies are used to determine the capacity of the bridge in its current condition.

Deficiency Qualification Describe the seriousness of a deficiency. For example:

- Crack sizes – record lengths, widths, and depth
- Section loss – record the remaining section dimensions (when reporting section loss, it is important to document the section remaining rather than trying to estimate the percentage of section loss)
- Deformation – record the amount of misalignment

Deficiency Quantification Describe the quantity of a deficiency. For example:

- Spalling – 2 feet x 3 feet x 2 inches deep
- Scaling – 4 feet high by full abutment width
- Delamination – 1 foot x 6 inches
- Decay – 2 feet x 2 feet x 3 inches deep

Deficiency Location

The exact position of the deficiency on the element or member is required if load capacity analysis is to be performed. For example:

- Left side of web, top half, 3 feet from north bearing
- Top of top flange, from 3 feet to 6 feet west of Pier 2

The accuracy of the load capacity analysis depends on precise location information for deficiencies:

- Bending moment – Maximum positive moment occurs at or near midspan. Maximum negative moment occurs at the intermediate supports if the structure is continuous.
- Shear/bearing – Shear is maximum at or near the supports. Bearing is maximum at the supports.
- Axial compression members – The capacity of the member to resist compressive forces is reduced by any deformation or change in cross section. The potential capacity reduction is not dependent on where on the member the deficiency is located. All segments are critical.
- Axial tension members – These members experience a reduction in capacity through loss of section or from cracking. As with the axial compressive members, tensile members are equally susceptible regardless of the location of the deficiency.
- Combinations – While axial members are critical at all locations, it is not always apparent which members are loaded only in an axial direction. In fact, due to the dead load of the member itself, most are not. Other factors can also contribute to bending forces that will create varying moments, shears, compression, and tension areas within a member that is primarily axial. Because of this, identify the exact position of the deficiencies in all members using reference points, regardless of the forces acting on the member.

Locating a deficiency may include tying it to an established permanent reference. Avoid using references that can change over time.

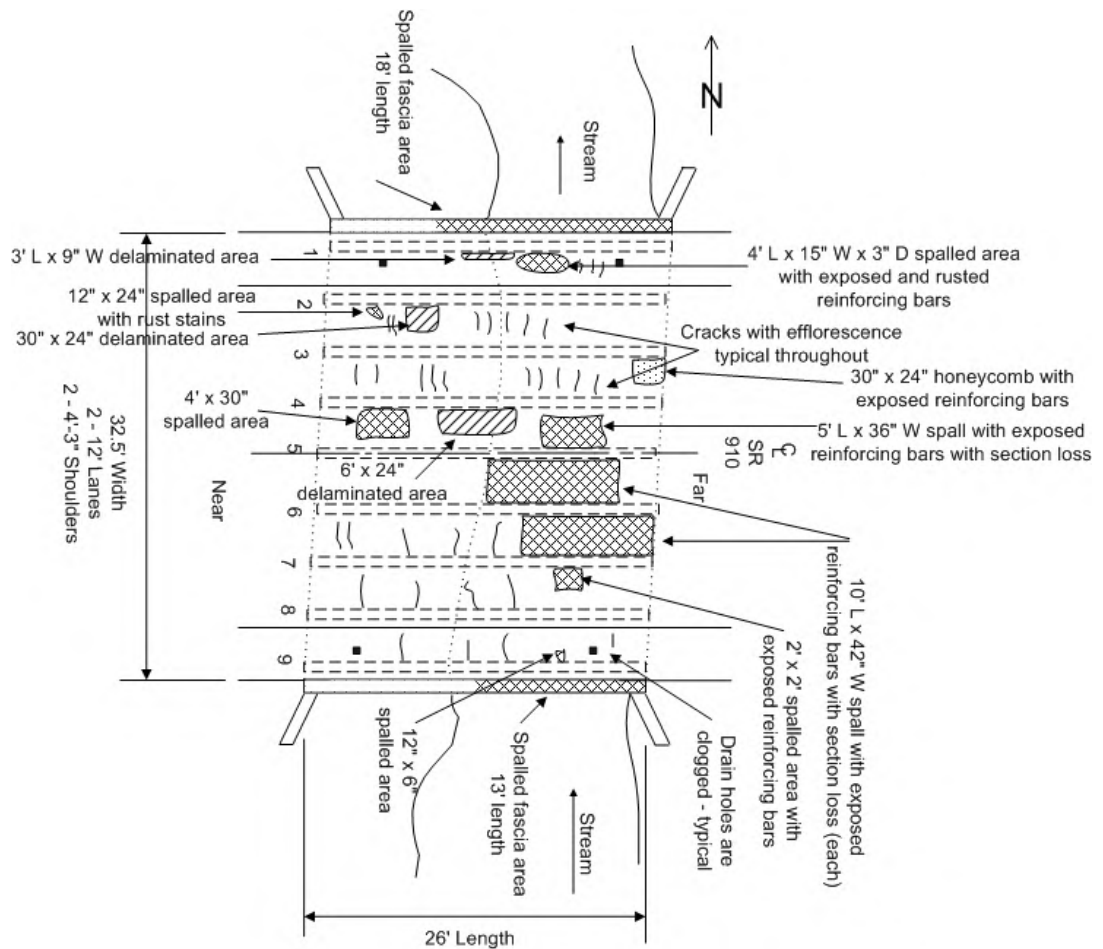
Some examples of proper referencing include:

- 7 feet-3 inches from fixed bearing on Beam 3 at Abutment 1
- 3 feet-1inch from west corner of Abutment 2
- 2 feet-6 inches below bridge seat on south face of Column 1, Pier 2

Reference points to avoid, since these locations vary between inspections:

- Expansion rocker faces
- Ground levels, especially those that may be exposed to water
- Water levels

When documenting the deficiency locations on the deck, include the condition of deck and haunch, expansion joints, construction joints, curbs, sidewalks, parapets, and railings with the deck sketches (see Figure 4.4.20).



ADT = 5956 in 2005
 Speed Limit = 45 mph

Plan View

Figure 4.4.20 Sample Deck Inspection Notes

When documenting the deficiency location of the superstructure, sketch the superstructure units in plan view and elevation, or cross section if necessary. Items to be inspected include bearings, main-supporting longitudinal members, floorbeams, stringers, bracing, and diaphragms (see Figure 4.4.21).

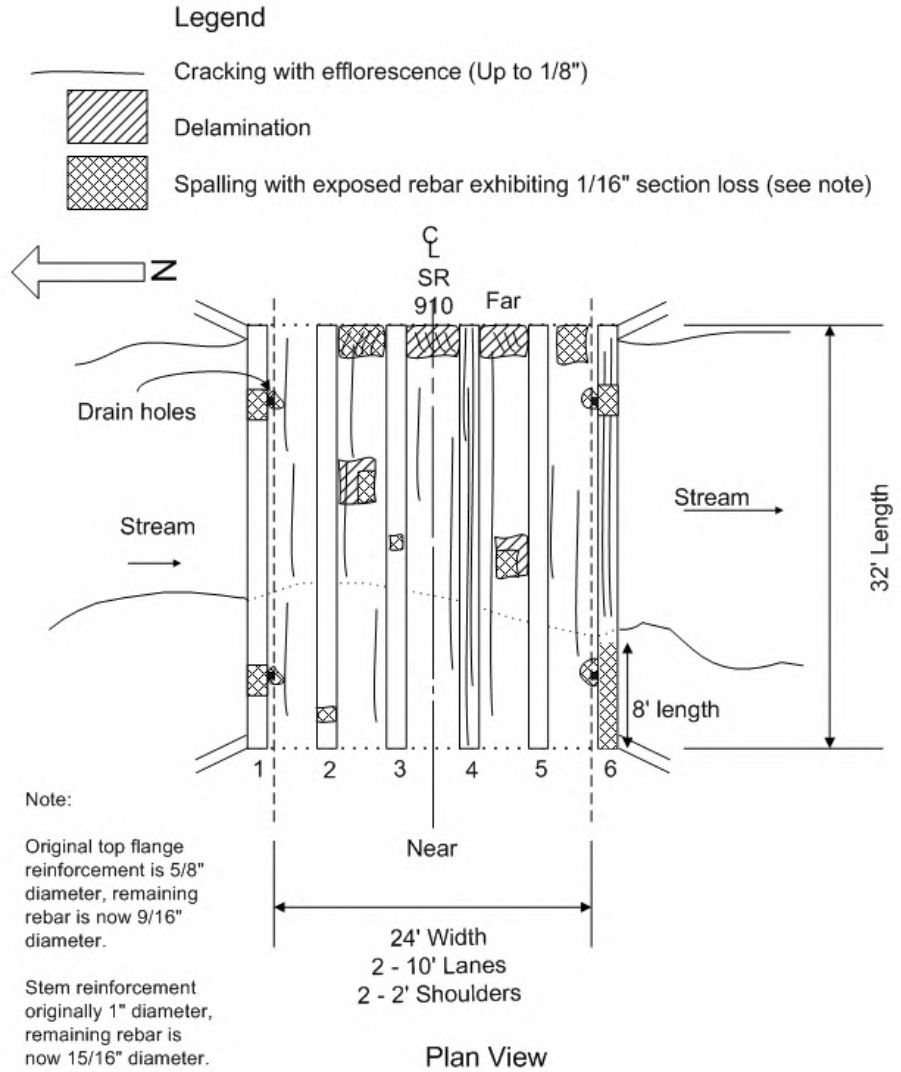


Figure 4.4.21 Sample Superstructure Inspection Notes

Include sketches or drawings to describe the condition of each substructure unit (see Figure 4.4.22). In many cases, it is sufficient to draw typical units that identify the principal elements and deficiencies of the substructure. Identify each element of the substructure unit so that they can be cross referenced to the notes or sketches. Items to be identified include piling, footings, vertical supports, lateral bracing of members, and caps.

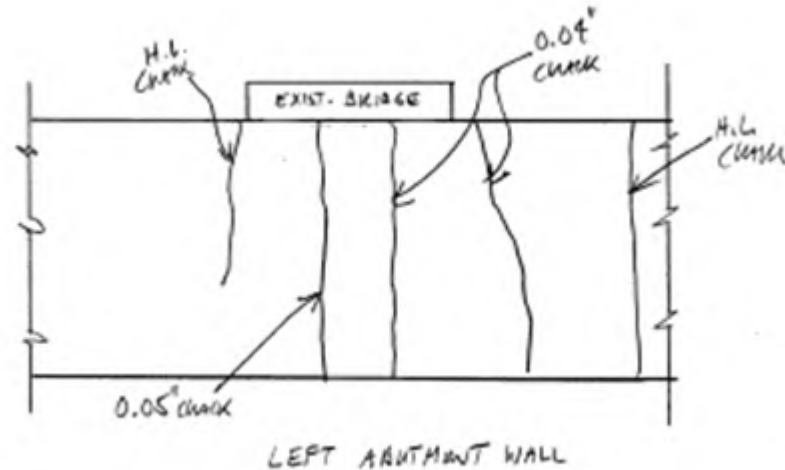


Figure 4.4.22 Sample Substructure Inspection Notes

Include sketches or drawings to describe the condition of the channel (see Figure 4.4.23). Streambed materials, alignment, condition of the banks, and the condition of the bottom of the waterway (including scour holes) are included in the sketch.

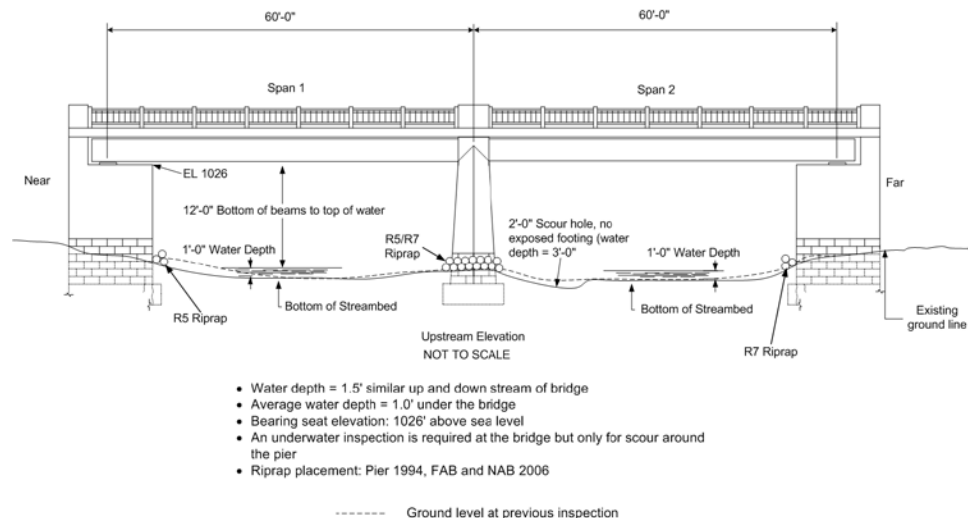


Figure 4.4.23 Sample Channel Inspection Notes

Summary of Findings

Report all deficiencies, no matter how minor they may seem. Be as descriptive as necessary to report not only the severity of the deficiency but the location as well. This will be described in further detail later in this topic. When reporting deficiencies, be objective and do not use terms such as “dangerous” or “hazardous”.



SITE DATA
 Form A

Form D-450A

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

1A09 Inspection Status: _____
7A02 Team Leader: _____
7A03 Inspection Type: _____
7A05 Inspected By: _____

Structure Description

5A08 FHWA Facility Carried: _____
5A07 Features Intersected: _____
5A09 Location: _____
5C01 Roadway Name: _____
5A06 City / Borough Name: _____

Structure Type

<p>Main</p> <p>6A26 Material Makeup: _____ 6A27 Physical Makeup: _____ 6A28 Span Interaction: _____ 6A29 Structural Config: _____</p>	<p>Approach</p> <p>6A26 Material Makeup: _____ 6A27 Physical Makeup: _____ 6A28 Span Interaction: _____ 6A29 Structural Config: _____</p>
---	---

Sign Information

ID01	ID02	ID03	ID06	ID04	ID07	ID05	
Type of Sign	Sign Needed	Sign Message	Near Adv	Bridge Site Near	Bridge Site Far	Far Adv	Comments
0 - Bridge	_____	_____	_____	_____	_____	_____	_____
1 - Bridge Weight Limit	_____	_____	_____	_____	_____	_____	_____
2 - Except Combinations	_____	_____	_____	_____	_____	_____	_____
3 - One Truck at a Time	_____	_____	_____	_____	_____	_____	_____
4 - Vertical Clearance On	_____	_____	_____	_____	_____	_____	_____
5 - Vertical Clearance Under	_____	_____	_____	_____	_____	_____	_____
6 - One Lane Bridge	_____	_____	_____	_____	_____	_____	_____
7 - Narrow Bridge	_____	_____	_____	_____	_____	_____	_____
8 - Hazardous Clearance	_____	_____	_____	_____	_____	_____	_____
9 - Other	_____	_____	_____	_____	_____	_____	_____

Report Version Date:

Page 1 of 3

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450



SITE DATA
Form A

Form D-450A

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Features Intersected

6C02		5C03		5C06		5C29		4A20		4A19		6C18		6C19		6C20		6C21		6C22		6C23		6C24		6B17		
SR	Seg	SR ID	On/Under	Skew Angle	Dir	NHS	Min Left	Lat Right	CI Left	Tot Left	Hor Right	CI Left	Min Left	Vrt Right	CI Left	Rdwys Right	Vert Left	CI Over 10ft	Right	VT Sign	ADT							

6B15 Design Exceptions: _____
6A51 Sub Latent Problem: _____
6A50 Sup Latent Problem: _____

Deck Geometry

Table Used for Appraisal: _____

Controlling Values

5C10 ADT: _____
5C27 Bridge Road Width: _____
4A10 Appraisal: _____
 Notes: _____

4A11 Underclr Appr: _____
6B13 Controlling Vertical: _____
 Controlling Lateral: _____

Traffic Safety Features

Feature Type	IA01 Location	IA02 Adequacy Rating	IA03 Description	5C08 Posted Spd Lmt (mph)
1 - Railing	_____	_____	_____	_____
Comment:	_____			
2 - Transition	_____	_____	_____	_____
Comment:	_____			
3 - Approach Guiderail	_____	_____	_____	_____
Comment:	_____			
4 - Approach Railend	_____	_____	_____	_____
Comment:	_____			

Report Version Date:

Page 2 of 3

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



DECK AND SUPER STRUCTURE DATA
 Form B

Form D-450B

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Deck Wearing Surface

<p>Main</p> <p>5B02 Type of Wearing Surface: _____</p> <p>5B03 Type of Memb. Water-Proof: _____</p> <p>5B04 Deck Corrosion Protection: _____</p> <p>6A33 Thickness: _____</p> <p>6A34 Date Recorded: _____</p> <p>6B40 Condition Rating: _____</p> <p>IC02 Dk WS Notes: _____</p>	<p>Approach</p> <p>6A30 Type of Wearing Surface: _____</p> <p>6A31 Type of Memb. Water-Proof: _____</p> <p>6A32 Deck Corrosion Protection: _____</p> <p>6A33 Thickness: _____</p> <p>6A34 Date Recorded: _____</p>
---	--

Expansion Joints **6A41** Number of Expansion Joints: _____

	VD25 Joint Type	VD26 Movement Class	VD27 Manufacture Code

Deck

1A01 Condition Rating: _____

6B07 Est. Spall Delamination: _____ **6B08** Date: _____

6B10 Est. Chloride Content: _____ **6B11** Date: _____

1A07 Unrepaired Spalls: _____

Deck Top: _____

Deck Underside: _____

Deck Drainage: _____

Expansion Joints: _____

Deck Notes: _____

Report Version Date:

Page 1 of 2

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



DECK AND SUPER STRUCTURE DATA
Form B

Form D-450B

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Superstructure

1A04 Condition Rating: _____

Narrative: _____

Girders/Beams: _____

Floorbeams: _____

Stringers: _____

Diaphragms: _____

Truss Members: _____

Bearings: _____

Drainage System: _____

Report Version Date:

Page 2 of 2

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



ABUTMENT DATA
Form C

Form D-450C

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

1A02 Substructure Condition Rating: _____

Notes: _____

Near Abutment

Backwall: _____

Bridge Seats: _____

Cheekwalls: _____

Stem: _____

Wings: _____

Footing: _____

Piles: _____

IN20 Scour / Undermine: _____

Settlement: _____

Embank Slope-Wall: _____

Wall Drainage: _____

Far Abutment

Backwall: _____

Bridge Seats: _____

Cheekwalls: _____

Stem: _____

Wings: _____

Footing: _____

Piles: _____

IN20 Scour / Undermine: _____

Settlement: _____

Embank Slope-Wall: _____

Wall Drainage: _____

Report Version Date: _____

Page 1 of 1

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



PIER DATA
 Form D

Form D-450D

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

4A21 Navigational Control Controls Exist: _____

4A22 Vert Clearance: _____
4A24 Lift Vertical: _____
4A23 Horz Clearance: _____
4A07 Pier Protection: _____

Pier Details

5D02 Pier/Bent Number: _____ **IN20** Scour / Undermine: _____

Condition Summary: _____

Bridge Seats: _____

Cheekwalls: _____
 Columns/Stems: _____
 Settlement: _____

5D02 Pier/Bent Number: _____ **IN20** Scour / Undermine: _____

Condition Summary: _____

Bridge Seats: _____

Cheekwalls: _____
 Columns/Stems: _____
 Settlement: _____

5D02 Pier/Bent Number: _____ **IN20** Scour / Undermine: _____

Condition Summary: _____

Bridge Seats: _____

Cheekwalls: _____
 Columns/Stems: _____
 Settlement: _____

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



ELEMENT DATA
 Form E

Form D-450E

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

6B03 Inventory Item Review Recommended: _____

IC01 Notes: _____

Element Details

5D02 Span: _____ **5D04** Span Type: _____

1B01 Element ID: _____ Inspect by Each: _____

Environment: _____ **1B05** Scale Factor Measurement: _____

Description: _____

1A10 Today QTY: _____ **1A11** Cond State 1 QTY: _____ **1A11** Cond State 2 QTY: _____

1A11 Cond State 3 QTY: _____ **1A11** Cond State 4 QTY: _____ **1A11** Cond State 5 QTY: _____

Condition: _____

Report Version Date:

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



FRACTURE CRITICAL
Form F

Form D-450F

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Main

6A44 Group: _____
Critical Rating Factor: _____
Total Critical Rating Factor: _____

Structure Type

6A26 Material Makeup: _____
6A27 Physical Makeup: _____
6A28 Span Interaction: _____
6A29 Structural Config: _____

Approach

6A44 Group: _____
Critical Rating Factor: _____
Total Critical Rating Factor: _____

Structure Type

6A26 Material Makeup: _____
6A27 Physical Makeup: _____
6A28 Span Interaction: _____
6A29 Structural Config: _____

Fracture Critical Details

IF01 Location: _____ **IF02** Type: _____ **IF05** FC Stress Category: _____
IF03 Member: _____
IF04 Member Detail: _____
IF06 Notes: _____

Report Version Date:

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



UNDERWATER INSPECTION
 Form G

Form D-450G

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

IU00a UW Reviewer Action: _____
IU00b Reviewer Comments: _____

IU02 Number of Units: _____ **IU01** Recalculate SCBI: _____
IU03 SCBI Source: _____ **4A08** SCBI: _____

Overall SCBI: _____ SAR: _____
IU06 Streambed Material #1: _____
IU06 Streambed Material #2: _____
IU07 Notes: _____

Current Countermeasures

CM Num	IU21 Type	IU22 Location	IU23 Condition	IU24 Subunit

Possible Countermeasures

PCM Num	IU25 Location	IU26 Work Candidate

SAR Calculation Data

IU08 Debris Potential: _____
IU09 Trapping Potential: _____
IU10 Pressure Flow: _____
IU11 NAB Location: _____ **IU12** FAB Location: _____

US Left Wingwall
IU13 Presence: _____ **IU14** Condition: _____

US Right Wingwall
IU15 Presence: _____ **IU16** Condition: _____

Horizontal Debris Blockage
IU17 Start: _____ **IU18** End: _____

Vertical Debris Blockage
IU19 Start: _____ **IU20** End: _____

Report Version Date:

Page 1 of 2

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



UNDERWATER INSPECTION
 Form G

Form D-450G

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Sub Unit OSA Data

Observed Scour Rating Components

IN01	IN12	IN13	IN14	IN15	IN19	IN04	IN05	IN06	IN07	IN08	IN09	IN10	IN11	IN03
Sub Unit	Abut Type	Found Type	Found Type	Strmbd Mat	Move Ind	Chg Since Last Insp	Scour Hole	Debris Potential	Scour-ability	Opening Adeq. / Channel	Sediment	Alignment	Velocity/ Stream Slope	Observed Scour Rating

Other Subunit Details

IN01	IN16	IN18	IN17	IN20	IN21	IN02	IN22	IN23		
Sub Unit	UW Insp Type	Water Dept	Observed Scour Depth	Scour Undermine	Counter-measures	Info from Current Insp	100 yr Flood Calc Scour Depth	500 yr Flood Calc Scour Depth	SCBI Code	SAR

IN24 Notes: _____

IN24 Notes: _____

IN24 Notes: _____

IN24 Notes: _____

Underclearance

IL09 Origin Description: _____
IL10 Horizontal: _____
IL11 Vertical: _____
IL12 Notes: _____

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



CULVERT DATA
 Form H

Form D-450H

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

1A03 Culvert Condition Rating: _____
 Notes: _____

5B18 Length of Culvert Barrel: _____

#	Opening Type	Length	Min Fill Height	Max Fill Height	Eff Width

Top Slab: _____
 Barrel: _____
 Floor/Paving: _____
 Headwall: _____
 Wings: _____
 Settlement: _____
 Debris: _____

Report Version Date:

Page 1 of 1

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



CHANNEL AND WATERWAY DATA
Form J

Form D-450H

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Channel

1A05 Channel/ Channel Protection Cond. Rating: _____

Channel: _____

Banks: _____

Streambed Movements: _____

Debris, Vegetation: _____

River Control Devices: _____

Embank/Strmbed Contr: _____

Drift, Other: _____

Waterway Adequacy

1A06 Appraisal Code: _____

Notes: _____

IL02 Overtop Risk: _____

IL03 Traffic Delay: _____

5C22 Functional Class: _____

High Water Mark

IL05 Elevation: _____ **IL06** Date: _____ **IL07** New High Water Mark: _____

Notes: _____

Report Version Date:

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



PAINT, STRUCTURE APPRAISAL AND LOAD RATINGS
 Form K

Form D-450K

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Paint Condition

6B36 Paint Cond Rating: _____ **6B37** Ext of Paint Cond: _____
6B35 New Paint: _____
 Int Beam / Gird: _____
 Fascias: _____
 Splash Zone: Truss Gird: _____
 Truss: _____
 Bearings: _____
 Other: _____

4B03 Bridge Cap. Appraisal: _____
6B16 Controlling: _____
4A09 Struct Cond Appraisal: _____

Structure Condition Appraisal Based on _____

Load Ratings

4B15 Load Rating Review Recommended: _____
 Due To: _____
 Calculation Date: _____
 Rating Approval Date: _____

Load Rating Details

	IR10	IR11	IR05	IR06	IR07	IR16	IR14	IR15	IR13	IR12
	IR	OR	NBI	RTNG	CONT	ANALYSIS	AASHTO	AASHTO	OPR	INV
LOAD	LOAD	LOAD	IND	ANAL	MEM	ENGINEER	MANUAL	SPEC	GOV	GOV
TYPE	LOAD	LOAD	IND	METH	TYPE	ENGINEER	YEAR	YEAR	CRITERIA	CRITERIA

Notes: _____

 Notes: _____

 Notes: _____

 Notes: _____

Report Version Date:

Page 1 of 1

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Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)



INSPECTION ADMINISTRATION
 Form P

Form D-450P

5A01 SR ID: _____ **5A03** BMS Ref: _____ **7A01** Inspection Date: _____

Current Inspection

7A03 Primary Type: _____

7A06 Types of Inspections Performed:

NBI Underwater Element Fracture Critical Other Special

Inspection Man Hours

6B26 NBI Crew: _____ **6B30** Underwater: _____

6B28 Fracture Critical: _____ **6B29** Other 1: _____

6B27 Crane: _____ **6B31** Other 2: _____

Inspection Cost (in hundreds)

6B32 Engineering: _____ **6B33** Rigging: _____

6B34 Office: _____

Special Equip Used:

6B12 Temperature: _____ **6B09** Weather: _____

6B03 Inventory Review Recommended: _____

Change Notes: _____

Inspection Team

7A04 Inspected By: _____

7A02 Team Leader: _____

6B23 Team Member: _____

6B24 Hired By: _____

6B25 Insp Contract Num: _____

2A02 Inspection Notes: _____

Next Inspection

7A14 Next Inspection By: _____

6B20 Next Insp Type: _____

Schedule

Insp Types	7A09 Required	7A09 Frequency	7A10 Next Date
NBI	_____	_____	_____
Fracture Critical	_____	_____	_____
Underwater	_____	_____	_____
Other Special	_____	_____	_____
Element	_____	_____	_____
Crane	_____	_____	6B18 _____

6B01 Special Insp Type: _____

Estimated Inspection Man Hours

7A12 NBI Crew: _____ **7A17** Underwater: _____

7A15 Fracture Critical: _____ **7A16** Other 1: _____

7A13 Crane: _____ **7A18** Other 2: _____

Report Version Date: _____ Page 1 of 1

This document includes structure safety inspection information that is confidential pursuant to 65 P.S. §66.1 et seq., 75 PA. C.S. §3754 and 23 U.S.C. §409 and may not be disclosed or used in litigation.

Figure 4.4.24 Example Inspection Form – PennDOT Form D-450 (continued)

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Topic 4.5 Critical Findings

4.5.1

Definition

A critical finding are a structural or safety related deficiency that requires immediate follow-up inspection or action.

A structure related deficiencies can interrupt the load path, not allowing loads to be transferred as designed. This can cause surrounding elements to become overstressed or unstable, potentially leading to partial or total collapse of the structure. Critical findings may also be non-structural deficiencies which jeopardize the safety of motorists or pedestrians.

4.5.2

Procedures

As stated in the NBIS regulations, each state or federal agency is required to "establish a statewide procedure to assure that critical findings are addressed in a timely manner." Although specific procedures vary among agencies, general steps must be taken to assure that critical findings are identified and resolved as quickly and efficiently as possible. The viable options available are permanently repair, temporarily repair or restrict loads on the bridge.

Currently, states employ two approaches to coding condition items when localized areas of severe deterioration are encountered. Some will account for the severity of a localized area of deterioration by lowering the condition rating of an entire component. The component condition rating is adjusted after the deteriorated area is improved (i.e., rating may rise if physical improvements are made, or may stay the same if the bridge is posted for load restrictions and/or supported with temporary shoring). FHWA recognizes this approach when the severity of the localized deterioration affects the load-carrying capacity of the component.

Other states rate to the general condition regardless of the severity of a localized area of deterioration. This approach relies heavily on ensuring that critical findings are addressed in a timely manner regardless of the component condition rating value. If the localized area of severe deterioration is not improved following the critical finding follow-up process, the component rating may need to be lowered to account for the severity of the deterioration if structural capacity is affected.

Either approach to coding the condition items results in the same ultimate outcome, i.e. critical inspection findings are addressed to allow continued safe use of the bridge. Component ratings eventually reflect the overall condition of the component. If the approach is to consider both the severity and extent of a component's deterioration in rating each component at the time of inspection (or up to 90 days after the inspection as required by the NBIS), there cannot be any assumptions about future improvements made to a localized area. Only if an improvement is made, the rating should then be raised as appropriate. If the improvement is made within 90 days of the inspection, there is no need to consider the localized deterioration in the rating.

Critical findings / critical follow-up report categorical contents with the documented status:

1. Bridges that have critical findings in the process of being addressed.
2. Bridges with work scheduled but not started yet.
3. Bridges that have no plan in the works.
4. Critical Finding is scour related.

Procedures for Inspectors

Upon identifying a potential critical finding, immediately report the deficiency to the appropriate agency official, bridge owner, or governing authority. For most agencies, a verbal notification is required soon after identifying the potential critical deficiency.

In addition to a verbal notification, agencies require immediate written notification of the potential critical finding. This notification is often presented in a standardized hardcopy or electronic format (see Figures 4.5.1 and 4.5.2), and is submitted soon after the verbal notification for most agencies. The written notification serves to document the critical finding by describing the extent of the deficiency complete with notes, photographs, sketches and drawings, measurements, possible causes, and recommendations for repair. Temporary actions may also be taken at this time to safeguard the public until proper repairs can be completed. These actions may include:


- Load posting
- Traffic restrictions from the damaged area
- Speed restrictions
- Temporary lane closure
- Temporary shoring
- Complete bridge closure

After submittal of the written report, the finding will be assessed and the severity determined along with a proposed repair strategy or plan of action. In accordance with NBIS regulations, the agency is also required to notify the FHWA of the critical finding. Public works officials or law enforcement may also be contacted as needed.

**Missouri Department of Transportation
Critical Inspection Finding
State System**

Bridge _____ District _____ County _____ Route _____ AADT _____ Location _____ Inspector _____ Inspection Date _____
Reason for Critical Inspection Finding report: Be specific about deficiencies. Attach Photographs.
Inspector's Immediate Recommendations: <input type="checkbox"/> Immediate Closure Required <input type="checkbox"/> Immediate Blocking/Shoring Required <input type="checkbox"/> Reduce traffic to one-lane. Carry traffic on <input type="checkbox"/> NB <input type="checkbox"/> SB <input type="checkbox"/> EB <input type="checkbox"/> WB lane <input type="checkbox"/> Other: _____
Immediate Notification: <input type="checkbox"/> State BM Engr <input type="checkbox"/> Supv Bridge Insp Engr <input type="checkbox"/>
MoDOT Action Plan by Bridge Maintenance and the District: _____ Date: _____
Follow-up Actions: _____ Completion Date: _____

Figure 4.5.1 Missouri DOT Critical Inspection Finding Form

 Washington State Department of Transportation		Critical Damage - Bridge Repair Report	
Agency Name		Charge Code	Bridge Name
Structure Identifier	Bridge Number	Bridge Location (Longitude/Latitude)	
Inspector (Print Name)		Inspector's ID Number	Inspection Date
Describe Deficiency			
Describe Recommended Repair			
Anticipated Date of Completion	Submitted By (Print Name)		Date Submitted
Describe Work Done			
Date of Completion	Submitted By (Print Name)		Date

DOT Form 140-151 EF
6/98

Figure 4.5.2 Washington State DOT "Critical Damage - Bridge Repair Report"

**Office Priority
Maintenance
Procedures**

Agencies establish priority maintenance procedures and prioritization criteria to help facilitate maintenance work plan strategies. Most agency systems utilize between three and five different prioritization levels ranging from general housekeeping and routine repairs to critical findings requiring immediate action. Examples of agency priority maintenance procedures are listed below in the order of most critical to least critical, with a description of each level.

Oregon Department of Transportation (ODOT)

- "Significant" – Severe deficiency to a primary bridge element that requires complete or partial closure of the bridge, or an immediate load restriction of the bridge.
- "Critical" – Serious deficiency to a primary bridge element that needs repair to prevent the bridge from being load posted.
- "Urgent" – Traffic safety related concern that does not jeopardize the reliability of the transportation system, protection of public investments, or maintenance of legal federal mandates.
- "Routine/Schedule" – Minor to moderate deficiency to a primary bridge element or moderate to major deficiency to a secondary element.
- "Monitor" – Non-structural housekeeping repairs such as cleaning the deck and drainage systems.

North Carolina Department of Transportation (NCDOT)

- "Critical Finding" – Severe deficiency to a primary bridge element that could cause partial or complete collapse or a safety feature deficiency that may jeopardize the safety of the public.
- "Priority Maintenance need" – Serious deficiency that may lead to load posting and/or bridge closures if left untreated.
- "Routine Maintenance need" – Minor to moderate deficiencies to primary or secondary bridge elements or non-structural housekeeping repairs such as cleaning the deck and drainage systems.

Pennsylvania Department of Transportation (PennDOT)

- "0 – Critical" – Severe deficiency to a primary bridge element that could directly or indirectly cause partial or complete structure collapse or a safety feature deficiency that may result in loss of vehicle operator control or failure to contain errant vehicles on the bridge deck.
- "1 – High Priority" – Serious deficiency to a primary bridge element that may lead to load posting and/or bridge closures. If left untreated, the deficiency may also jeopardize public safety.
- "2 – Priority" – Advanced deficiency on a primary bridge element or appurtenance that if left untreated, may lead to continuing deterioration, load posting, or partial or complete bridge closures.
- "3 – Schedule" – Minor deficiency to a primary bridge element or appurtenance that may continue to deteriorate if left untreated.
- "4 – Program" – Note-worthy problem on a primary bridge element, secondary element, or appurtenance that may lead to a documentation-worthy deficiency if left untreated.
- "5 – Routine" – Non-structural housekeeping maintenance that may lead to deterioration of primary and secondary structural members if left untreated.

Bridge Closing Procedure

In some situations, the bridge may need to be closed until the critical finding can be repaired. The decision to close the bridge may result from the nature of the critical finding upon initial discovery, an unacceptable timeframe in which the repairs are scheduled to be completed, or agency policy on critical findings.

For situations recommending closure of the bridge by the bridge inspector and/or bridge maintenance supervisor, follow established State or Federal Agency procedures. Examples of acceptable procedures include:

- Contact the Bridge Maintenance Supervisor about the recommended closing.
- Contact the Bridge Engineer about the recommended closing.
- If both the Bridge Maintenance Supervisor and Bridge Engineer are unavailable, contact the District or Division office about the recommended closing.

4.5.3

Examples of Critical Findings

FHWA guidance for a follow-up may include a procedure where the State promptly submits to the Division office a copy of inspection reports or recommendations for all on-system and off-system bridges that meet the following criteria:

1. Bridges with recommendations for immediate work on fracture critical members;
2. Bridges with recommendations for immediate correction of scour or hydraulic problems;
3. Bridges with condition ratings of 3 or less for the superstructure or substructure or appraisal ratings of 3 or less for waterway adequacy; and
4. Bridges with recommendations for immediate work to prevent substantial reduction in the safe load capacity.

Source: <http://www.fhwa.dot.gov/bridge/0650csup.cfm>

Many state agencies publish examples of critical findings for bridge inspectors. It should be noted that these lists are not all-inclusive or comprehensive and should only be used as guidance in determining whether or not a deficiency is a critical finding.

The critical findings listed below are organized by material type and application. These deficiencies represent excerpts obtained from several agencies' critical finding documentation

Timber

The following deficiencies represent examples of critical findings for timber:

- Through-loss in deck planks and broken planks in danger of breaking through.
- Primary structural members with collision damage that compromises the structural capacity (including severe section loss, full length horizontal cracking, and section loss to truss compression members producing

member buckling or severe flexural cracking).

- Primary structural members with multiple open cracks in high stress regions or crushing/decay that may lead to superstructure settlement.
- Crushed or broken nailer boards or broken joists.
- Piles and pier caps that have loss of bearing capacity or soil retention through crushing, decay, or insect damage.
- Substructure units with severe scour and undermining of the substructure foundation causing instability.

Concrete

The following deficiencies represent examples of critical findings for concrete:

- Section loss (thru-hole) subject to enlargement by traffic or deep spalls with exposed rebar in danger of holing through, creating a safety hazard to passing traffic.
- Prestressed girder with spalling and broken strands or 100% deterioration at critical high stress areas.
- Non-composite prestressed adjacent box beams with serious deterioration and existing strand loss, loss of camber or torsional cracking.
- Reinforced concrete girder or pier cap with spalling and broken main rebar or 100% deterioration, with more than one bar affected at the same location in the girder.
- Reinforced or prestressed concrete girder bearing area resulting in loss of bearing area and making girder subject to settlement.
- Reinforced concrete columns with spalling and rebar section loss causing the column to be subject to failure.
- Primary structural members with collision damage that compromises the structural capacity (including severed prestressing tendons, reinforcing steel that results in flexural cracking and negative beam camber, pier shafts, and columns).
- Concrete pier column or cap with significant structural cracking that is supporting a fracture critical bridge or fracture critical component.
- Falling concrete or concrete that is delaminated or partially detached and anticipated to fall, presenting a safety hazard to under-passing motorists and/or pedestrians.
- Bearing seats that are severely deteriorated or undermined.
- Sidewalk structural supports or walking surface with damage or deterioration presenting a hazardous condition to pedestrians.
- Substructure units with severe scour and undermining of the substructure foundation causing instability.

Steel

The following deficiencies represent examples of critical findings for steel:

- Steel members with deteriorated areas that have failed in buckling, crippling, more than 10% of the connectors in a connection are missing, etc., or which makes failure likely in the near future.
- Secondary structural members (diaphragms, bracing, etc.) with extensive section loss.
- Fracture critical members subjected to impact damage including gouging or tearing, perpendicular stress cracks in either the base metal or weld metal, parallel stress cracks resulting from out-of-plane distortions or poor weld details, and severe corrosion in girder flanges, webs, in truss members, or in gusset plates.
- Primary structural members with collision damage that compromises the structural capacity (including fractures, large gouges, significant twisting/kinking of beams, and section loss to truss compression members producing member buckling or severe flexural cracking).
- Primary structural member (non-FCM member) with a completely fractured tension member due to fatigue or vehicular collision.
- Pin and hanger systems in fracture critical members with severe deterioration or severe accumulation of debris or rust packing.
- Bottom flange cover plates with cracked welds at the end of a partial length welded cover plate for a steel multi-girder or steel floorbeam.
- Substructure units with severe scour and undermining of the substructure foundation causing instability.

Roadside Hardware or Safety Features

The following deficiencies represent examples of critical findings for traffic safety features:

- Bridge railing (bridge parapets, median barriers, or structure-mounted guardrail) with damage or deterioration that may prevent containment and/or redirection of errant vehicles traveling at the posted speed limit.
- Pedestrian railing that is missing or detached, allowing a pedestrian to fall off the structure.
- Guardrail connections to bridge railing, concrete barrier rebar, or guardrail that is detached and in close proximity or projecting into traffic with potential for impact.

Signs and Lighting

The following deficiencies represent examples of critical findings for signs and lighting:

- Load posting or vertical clearance signs that are missing, damaged, improperly located, or visually obstructed including relevant advance warning signs.
- Signs, traffic signals, or strain poles presenting a safety hazard to passing motorists and/or pedestrians due to extensively damaged, split or buckled sections, or with cracked welds at either pole/base connections or

member/member connections.

- Sign, traffic signal, or strain pole 4-bolt base plate connections with one or more loose nuts presenting a safety hazard to passing motorists and/or pedestrians.
- Signs with deteriorated or missing panel connectors, allowing sign to "flop" under wind loading that present a safety hazard to passing motorists and/or pedestrians.
- Lighting fixtures with split sections, buckled sections, significant section loss, and/or cracked welds at the pole/base connection that present a safety hazard to passing motorists and/or pedestrians.

Other

The following deficiencies represent other examples of critical findings:

- Expansion joints that are deteriorated, damaged, or loose which may present a safety hazard to passing traffic.
- Rocker bearings that are critically tilted either exceeding the acceptable amount of tilt or bearing on the outer one-quarter width of the rocker.
- Excessive debris and/or sediment buildup at the hydraulic opening for scour critical bridges or other bridges with unknown foundations.

4.5.4

Example Plans of Action

As previously mentioned, a statewide or Federal agency wide procedure must be established to assure that critical findings are addressed in a timely manner. The appropriate actions to be used for repair or mitigation of the critical finding must be quickly identified and efficiently carried out. The FHWA must be periodically notified of the actions that have been taken to resolve or monitor critical findings. It is the responsibility of Bridge Owners to implement procedures for addressing critical deficiencies including:

- Immediate critical deficiency reporting steps
- Emergency notification of police and the public
- Rapid evaluation of the deficiencies
- Rapid implementation of corrective or protective actions
- A tracking system to ensure adequate follow-up
- Provisions for identifying other bridges with similar structural details for follow-up inspections

Some agencies have very strict timeframes (3 to 7 *calendar* days) for developing and accepting plans of action. For circumstances involving immediate attention or a more detailed solution, it may be necessary to begin addressing the critical finding (through permanent or temporary work) prior to the 100% development and acceptance of the plan of action. Example plans of action are given below for Pennsylvania DOT (Figure 4.5.3) and Washington State DOT (Figure 4.5.4).

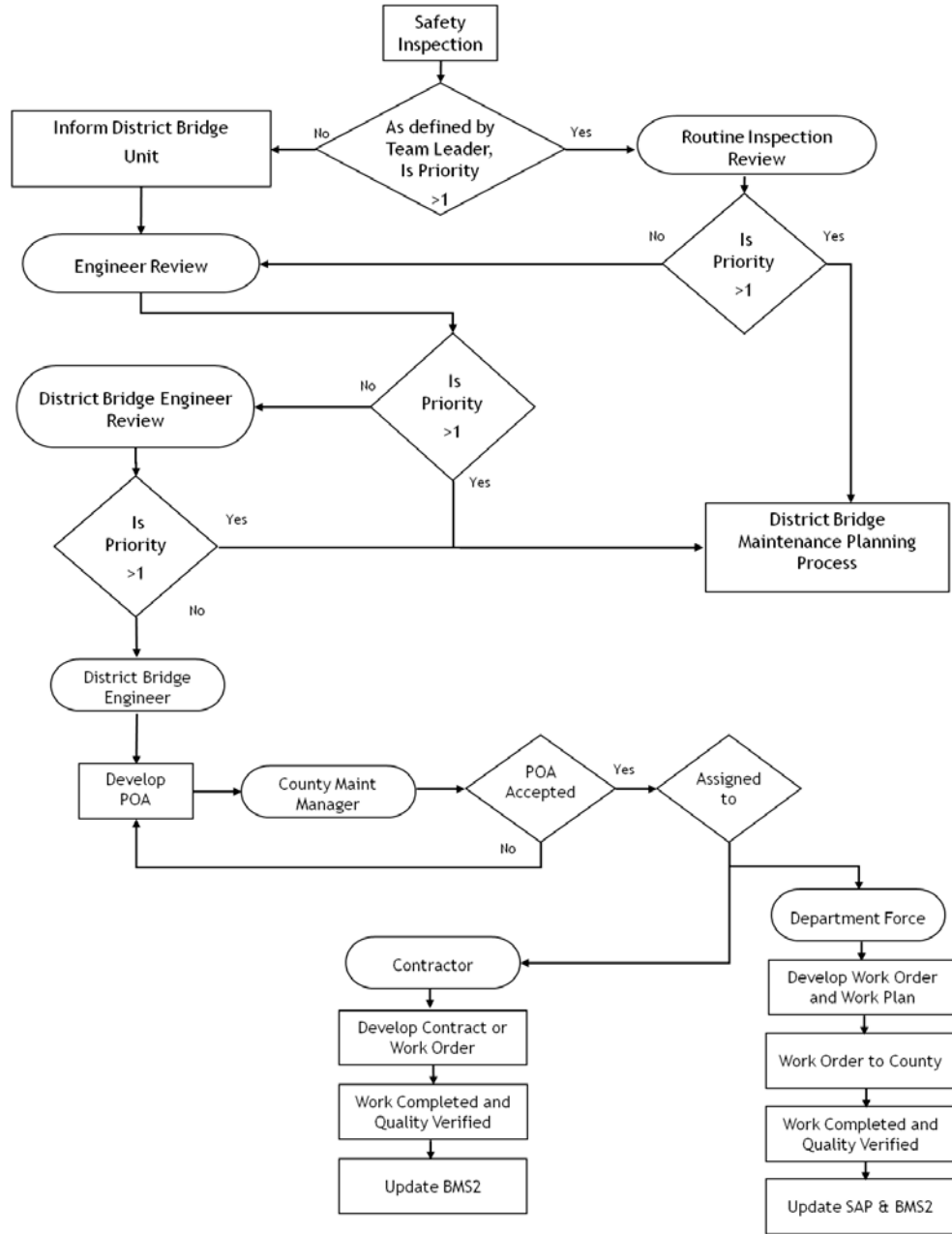


Figure 4.5.3 Pennsylvania DOT Critical and High Priority Maintenance Items – Flowchart for Plan of Action

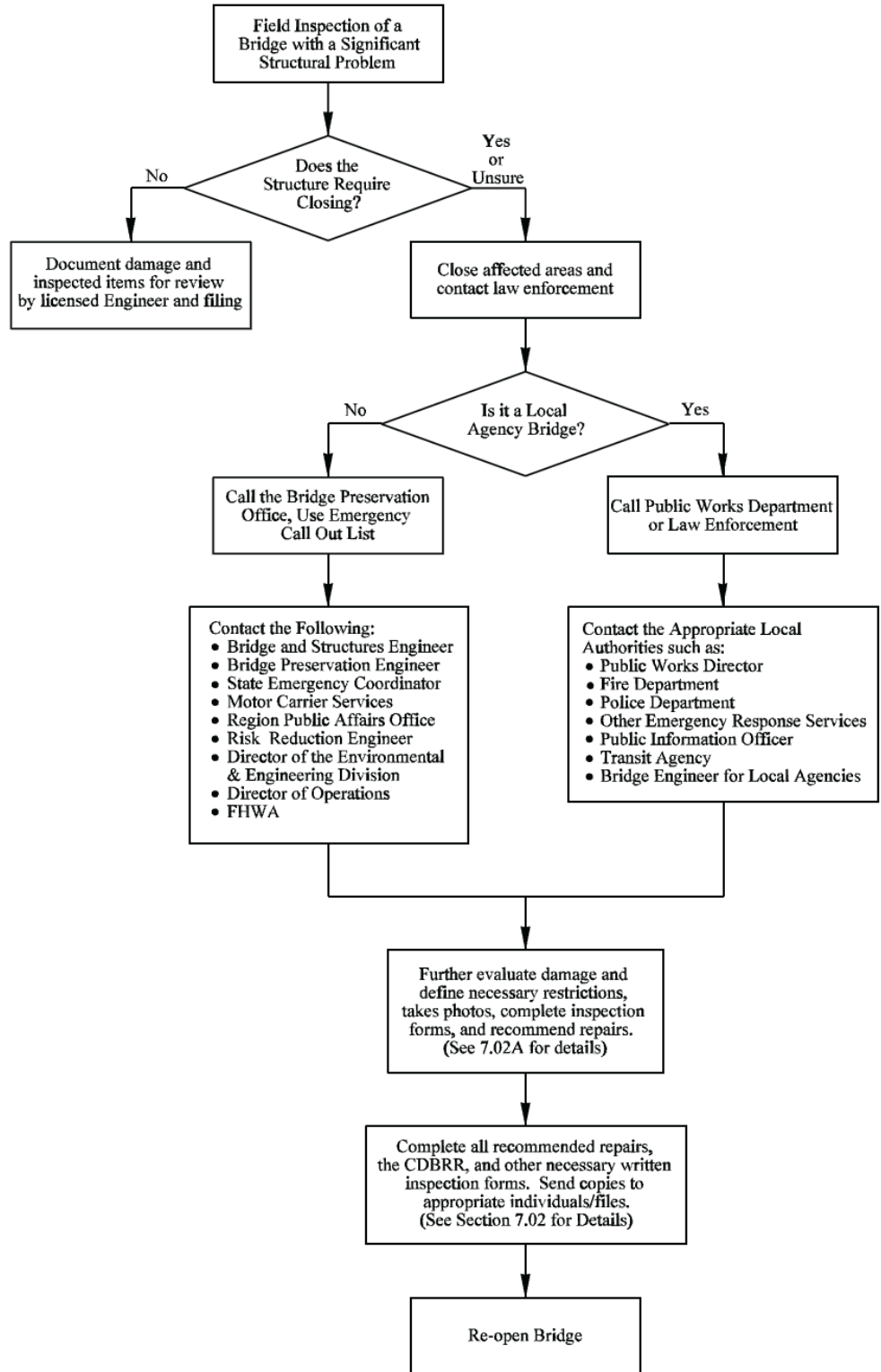


Figure 4.5.4 Washington State DOT Flowchart for Field Inspection Procedure

After the plan of action has been accepted, recommended repair work will then be performed and completed within a few days up to several weeks, depending on the individual agency's regulations. A post-repair report will be generated documenting all necessary work done to address the critical finding and the date of completion. A follow-up inspection will also be conducted to assess the condition of the repairs. The FHWA will be notified of the repair and post-repair progress.

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Topic 4.6 The Inspection Report

4.6.1

Introduction

The purpose of the bridge inspection reporting system is to have trained and experienced personnel record objective observations of all elements of a bridge and to make logical deductions and conclusions from their observations.

The bridge inspection report represents a systematic inventory of the current or existing condition of all bridge members and their possible future weaknesses. Moreover, bridge reports form the basis of quantifying the manpower, equipment, materials, and funds that are necessary to maintain the integrity of the structure.

A bridge inspection is not complete until an inspection report is finalized. The bridge inspection report documents all signs of distress and deterioration with sufficient precision so that future inspectors can readily make a comparison of condition. Bridge owners normally set the format to be used when preparing a bridge inspection report. A complete inspection report contains several parts, as outlined in this topic. A sample bridge inspection report is presented in Appendix A. Inspection reports are prepared for special inspections, which are conducted for checking a specific item where a problem or change may be anticipated. Even if no changes are evident, reports are still generated for each type of bridge inspection. Some bridge owners also request a special bridge inspection and report when planning a major rehabilitation.

4.6.2

Basic Components of a Comprehensive In-Depth Bridge Inspection Report

Table of Contents

The table of contents presents the general headings and topics of the inspection report in an orderly manner so that individual sections of the report can be found with ease. It generally follows the title page, and individual sections are listed with their corresponding starting page number.

Location Map

A map is normally included with a scale large enough to positively locate the structure. The bridge is clearly marked and labeled, and the map has a north arrow to aid with orientation. Some agencies may choose to use GPS coordinates or latitude/ longitude descriptions.

Bridge Description and History

The bridge description and history section of the report contains all pertinent data concerning the design, construction, and service of the bridge. The type of superstructure will generally be given first, followed by the type of abutments and piers or bents, along with their foundations. If data is available, indicate the type of foundation soil, maximum bearing pressures, and deep foundation capacities. The type of deck is also indicated.

The history of the bridge is from a structural standpoint and is developed from information obtained from design, construction and rehabilitation plans, previous inspection reports, maintenance records, discussions with maintenance crews and

local residents, and any other available source that offers pertinent information. Typical items included in the history narrative are:

- Historical flood frequencies and high water marks
- Maintenance measures and repairs
- Chronological record of conditions (in order to help determine a rate of deterioration of all bridge components and the channel). The agency establishes criteria for the number of bridge inspections kept on file.
- Reference drawings
- Photos, which would consist of a typical approach photograph showing the approach roadway, bridge and any load restriction signs, as well as an elevation/profile photograph showing upstream/downstream of the bridge. Other photographs, such as those conveying the condition of the bridge and its components, would be found in the Appendix or in the Inspection Results section of the inspection report.

Design Data

The design information includes a description of the following:

- | | |
|--------------------------------|----------------------------------|
| ➤ Skew angle | ➤ Railing and median |
| ➤ Number and length of spans | ➤ Year constructed/reconstructed |
| ➤ Span type and material | ➤ Number of traffic lanes |
| ➤ Total length | ➤ Design live loading |
| ➤ Bridge width | ➤ Waterway |
| ➤ Deck structure type | ➤ Other features intersected |
| ➤ Wearing surface | ➤ Clearances |
| ➤ Deck protection and membrane | ➤ Encroachments |
| ➤ Sidewalks | ➤ Alignment |

Construction Data

The construction history of the bridge includes the date it was originally built, as well as the dates and descriptions of any repairs or reconstruction projects. State what plans are available, where they are filed, and whether they are “design”, “as-built”, or “rehabilitation” drawings.

Service Data

The average daily traffic (ADT) count and the average daily truck traffic (ADTT) count are included, along with the date of record. This information is updated approximately every five years. Other service data to consider includes the year of ADT and ADTT, facility carried, functional classification, and bypass detour length and map. In addition, environmental conditions that may have an effect on the bridge, such as salt spray, industrial gases, bird droppings, and ship and railroad traffic, are noted in the report.

Executive Summary

The executive summary is a narrative presentation summarizing the inspection and analysis findings in regard to the qualitative condition and the load capacity of the bridge, along with an overview of recommendations. A typical executive summary identifies the bridge (e.g., name, number, and location) and the date of inspection. The executive summary presents any high priority repair items.

Inspection Procedures

The procedures used to inspect the bridge are documented in the inspection report. In most instances, it is advantageous to inspect structures in the same sequence as the load path (i.e., the deck first, then the superstructure, and finally the substructure). This manual is organized and presented for that sequence.

Many inspections cannot follow this sequence due to traffic and lane-closure restrictions. It is useful to document whatever sequence was used during the inspection. This information will be useful in planning future inspections and will also serve as a checklist to make sure that all elements and components were inspected. The following information is typically included:

- Equipment required (e.g., hammers and plumb bobs)
- Access equipment (e.g., rigging, ladders, and free climbing)
- Access vehicles (e.g., inspection cranes and bucket trucks)
- Traffic restrictions (e.g., lane closures, flagmen, and hours of operation)
- Permits required (e.g., railroad and Coast Guard)
- Inspection methods (e.g., visual, physical or advanced)
- Personnel (e.g., by name and classification)
- Special equipment (e.g., material testing and underwater inspection)
- Deviations from “hands-on” inspection of all areas
- Time required for inspection
- Channel profiles, cross sections and scour criticality

When structure plans are not in the bridge records and a load rating has not been calculated, it may be necessary to obtain field measurements to assist in the calculation of the load capacity of the structure.

Inspection Results

Provide narrative descriptions of the conditions both quantitative and qualitative, indicating the locations and the extent of the affected areas. Use agency-approved forms consistent with similar inspections. Note all signs of distress, failure, or defects with sufficient precision so that a deterioration rate can be determined. This is very important for determining estimated remaining life and an optimal preservation strategy. Take photographs in the field to show deficiencies and cross reference in the report or on forms where deficiencies are noted. Supplement written notes with sketches and photos to show location and physical characteristics of deficiencies, including a known object in the photograph for scale reference.

Note any load, speed, or traffic restrictions on the bridge. Indicate if the signs are missing or damaged. Take approach roadway photograph to confirm placement of load posting signs that includes the approach roadway, bridge and sign. Check for advanced warning signs. Include information about high water marks and unusual loadings. Note the weather conditions such as temperature, rain, or snow. Note all work or repairs to the bridge since last inspection. Verify or obtain new dimensions when improvement work has altered the structure. New streambed profiles and cross sections are taken to detect scour, channel migration, or channel aggradation and degradation. Note any channel restrictions (e.g. debris) that could impact stream flow and increase scour potential. State the seriousness and amount of all deficiencies at the bridge site.

Load Rating Summary

A summary of any load capacity rating analysis that has been performed is included in the report. The summary is presented in a table or chart. Governing load ratings are shown for both inventory and operating levels for all types of loadings used in the analysis. Identify the governing member for each rating. The governing member is the one that has the lowest capacity for a given type of loading.

For example, in a girder-floorbeam-stringer structure, Stringer three in Bay five may have the lowest capacity for carrying HS20 trucks, compared to all other stringers, floorbeams, or girders. The HS20 inventory and operating ratings for this stringer is reported, and it would be identified as the governing member.

Conclusions and Recommendations

A good inspection report explains in detail the type, severity and extent of any deficiency found on the bridge and points out any deviations or modifications that are contrary to the “as-built” construction plans. The depth of the report is consistent with the importance of the deficiencies. Not all deficiencies are of equal importance. For example, a crack in a prestressed concrete box beam which allows water to enter the beam is much more serious than a vertical crack in an abutment backwall or a spall in a corner of a slopewall.

The inspector’s experience and judgment are called upon when interpreting inspection results and arriving at reasonable and practical conclusions. Improper and misinformed conclusions will lead to improper recommendations. The inspector may need to play the role of a detective to conclude why, how, or when certain deficiencies occurred. Seek advice from more experienced personnel when you cannot confidently interpret the inspection findings.

The recommendations made by the inspector constitute the “focal point” of the operation of inspecting, recording, and reporting. The inspector reviews previous inspection recommendations and identifies any recommendations that have not been addressed, particularly if urgent. A thorough, well-documented inspection is essential for making informed and practical recommendations to correct or preclude bridge deficiencies.

All recommendations for preservation work, load rating, postings, and further inspection are included in this portion of the inspection report. Carefully consider the benefits to be derived from completing recommended work and the consequences if the work is not completed. List, in order of greatest urgency, any work that is necessary to maintain structural integrity and public safety. Recommendations concerning work are typically classified between three to five distinct prioritization levels, which range from the most severe or significant (critical) to a maintenance item that is considered routine or may only require monitoring (non-critical). The specific prioritization levels are set forth by each bridge-owning agency. Examples of agency priority maintenance procedures are listed in Topic 4.5.2.

The inspector decides whether a deficiency is a critical finding and needs immediate action using agency procedures. Usually this is easily determined, but occasionally the experience and judgment of a professional engineer may be required to reach a proper decision. A large hole through the deck of a bridge obviously needs attention, and a recommendation for immediate action is in order. Communicate the critical finding immediately and document actions taken in the

report. By contrast, a slightly deteriorated bridge bearing may not be critical. A condition such as this would appropriately call for a recommendation for a preservation action.

Typically, most work recommendations submitted by the bridge inspector will be in the category of non-critical work. The recommended work is carefully described in the report along with a cost estimate.

If not already described in the executive summary, the conclusions and recommendations section of the report summarizes the following:

- Overall condition
- Major deficiencies
- Load-carrying capacity
- Recommendations for:
 - Further inspection
 - Maintenance
 - Repairs
 - Painting
 - Posting
 - Rehabilitation
 - Replacement

Some state and local agencies designate separate personnel, not the inspector in the field, to prepare recommendations and cost estimates.

Report Appendices

To achieve maximum effectiveness of the inspection report, the report appendices contain any back-up information used to substantiate the inspector's findings, conclusions and recommendations. Typically, the appendices include photographs, drawings and sketches, and inspection forms (see Topic 4.4 for record keeping and documentation). Appendices may also include copies of any field notes used and specialist reports (e.g., underwater, nondestructive evaluation (NDE), and survey), or these documents may be referenced in the report. A load capacity rating analysis of the structure may also be incorporated into the report appendices. It is important to have the inspection report and all supplemental information, including report appendices, accurate with clear and concise descriptions or explanations.

Photographs

Photographs are a great asset to anyone reviewing reports on bridge structures. It is recommended that pictures be taken of any problem areas. Take pictures even if you think you can explain it completely in writing. It is better to take several photographs that may be considered unessential than to omit a photograph that could cause misinterpretation or misunderstanding of the report. At least two general photographs of every structure are provided in the appendix. One of these depicts the structure from the roadway, while the other photo is a view of the side elevation (see Figures 4.6.1 and 4.6.2). Captions are provided for each photograph. Photographs are numbered so that they can be referred to in the body of the report. Sketches may also be a substitute for missing as-built plans.



Figure 4.6.1 Near Approach - Toward Bridge



Figure 4.6.2 Downstream Elevation

Drawings and Sketches

Sketches and drawings needed to illustrate and clarify conditions of structural elements or serve as as-built plans are included or referenced. Sketches may be able to convey information not readily identified in a photograph (ie. remaining web thickness). Original drawings are very helpful during future investigations with determining the progression of defects and to help determine any changes and their magnitude. Drafting-quality plans and sketches, sufficient to indicate the layout of the bridge and bridge site, may be included as an appendix.

Some reports combine photographs and sketches or text boxes together to accurately describe and document a particular deficiency.

Inspection Forms

The inspection forms contain the actual field notes, as well as the numerical condition and appraisal ratings by the inspector. The inspection forms are normally signed by the inspection team leader. A complete SI&A form or equivalent is included in the appendix. Compare previous inspection forms to current conditions for inventory data accuracy.

Load Capacity Analysis

A load rating analysis is performed on the structure to determine the load-carrying capacity of the bridge. It includes the investigation of primary load-carrying members of the bridge. Such analysis is normally performed by engineers in the office, not by the inspector. Also, not all inspections require a new load rating analysis. A new load rating analysis is performed if the condition of the primary members has changed considerably since the last inspection. The report also includes recommendations for a new load rating analysis when maintenance or improvement work, change in strength of members, or dead load has altered the condition or capacity of the structure.

Field Inspection Notes

Include the original notes taken by the inspectors in the field or photocopies thereof in the appendix section of the report. The original field notes are source documents and as such are typically included in the bridge record.

Underwater Inspection Report

If an underwater inspection of the substructure has been performed, a separate report is usually prepared by the dive team. If applicable, include the underwater inspection report in the appendix or cross-reference the location of the report.

Material Testing Results

Material testing may be performed on a structure in order to determine the strength and properties of an unknown or suspect material. Include the testing lab's report in the appendix.

4.6.3

Basic Components of a Comprehensive Routine Inspection Report

Location Map

A map with a scale may be included to help positively locate the structure. Some agencies may choose to use GPS coordinates or latitude/longitude descriptions to supplement or replace the location map.

Inspection Procedures

The procedures used to inspect the bridge may be documented in the inspection report. For inspection reports that include the inspection procedures, it is advantageous to inspect structures in the same sequence as the load path (i.e., the deck first, then the superstructure, and finally the substructure).

As with in-depth inspections, some routine inspections cannot follow this sequence due to traffic and lane-closure restrictions. Therefore, it is useful to document whatever sequence was used during the inspection. This information will be useful in planning future inspections and will also serve as a checklist to make sure that all elements and components were inspected. The following information is typically included:

- Equipment required (e.g., hammers and plumb bobs)
- Access equipment (e.g., rigging, ladders, and free climbing)
- Access vehicles (e.g., inspection cranes and bucket trucks)
- Traffic restrictions (e.g., lane closures, flagmen, and hours of operation)
- Permits required (e.g., railroad and Coast Guard)
- Inspection methods (e.g., visual, physical or advanced)
- Personnel (e.g., by name and classification)
- Special equipment (e.g., material testing and underwater inspection)
- Deviations from “hands-on” inspection of all areas
- Time required for inspection
- Channel profiles, cross sections and scour criticality

When structure plans are not in the bridge records and a load rating has not been calculated, it may be necessary to obtain field measurements to assist in the calculation of the load capacity of the structure.

Inspection Results

The results of the inspection are documented within the inspection forms. Narrative descriptions of the conditions are typically not included for routine inspection reports. As with in-depth inspections, use agency-approved forms consistent with similar inspections. Note all signs of distress, failure, or defects with sufficient precision so that a deterioration rate can be determined. This is very important for determining estimated remaining life and an optimal preservation strategy. Take photographs in the field to show deficiencies and cross reference in the report or on forms where deficiencies are noted. Supplement written notes with sketches and photos to show location and physical characteristics of deficiencies, including a known object in the photograph for scale reference.

Note any load, speed, or traffic restrictions on the bridge. Indicate if the signs are missing or damaged. Take approach roadway photograph to confirm placement of load posting signs that includes the approach roadway, bridge and sign. Check for advanced warning signs. Include information about high water marks and unusual loadings. Note the weather conditions such as temperature, rain, or snow. Note all work or repairs to the bridge since last inspection. Verify or obtain new dimensions when improvement work has altered the structure. New streambed profiles and cross sections are taken to detect scour, channel migration, or channel aggradation and degradation. Note any channel restrictions (e.g. debris) that could impact stream flow and increase scour potential. State the seriousness and amount of all deficiencies at the bridge site.

Load Rating Summary

For routine inspections, a load rating may be conducted. If performed, a load rating summary is included in the report and may also be included on the inspection forms. The summary is presented in a table or chart. Governing load ratings are shown for both inventory and operating levels for all types of loadings used in the analysis. Identify the governing member for each rating. The governing member is the one that has the lowest capacity for a given type of loading.

Conclusions

A routine inspection report may or may not contain conclusions of the inspection. If conclusions are included, explain in detail the type, severity and extent of any deficiency found on the bridge and point out any deviations or modifications that are contrary to the “as-built” construction plans. The depth of the report is consistent with the importance of the deficiencies. Not all deficiencies are of equal importance.

The inspector’s experience and judgment are called upon when interpreting inspection results and arriving at reasonable and practical conclusions. Improper and misinformed conclusions will lead to improper recommendations. The inspector may need to play the role of a detective to conclude why, how, or when certain deficiencies occurred. Seek advice from more experienced personnel when you cannot confidently interpret the inspection findings.

Recommendations

Recommendations are made by the inspector that constitutes the “focal point” of the operation of inspecting, recording, and reporting. The inspector reviews previous inspection recommendations and identifies any recommendations that have not been addressed, particularly if urgent. A thorough, well-documented inspection is essential for making informed and practical recommendations to correct or preclude bridge deficiencies.

All recommendations for preservation work, load rating, postings, and further inspection are included in this portion of the inspection report. Carefully consider the benefits to be derived from completing recommended work and the consequences if the work is not completed. List, in order of greatest urgency, any work that is necessary to maintain structural integrity and public safety. Recommendations concerning work are typically classified between three to five distinct prioritization levels, which range from the most severe or significant (critical) to a maintenance item that is considered routine or may only require monitoring (non-critical). The specific prioritization levels are set forth by each bridge-owning agency. Examples of agency priority maintenance procedures are

listed in Topic 4.5.2.

The inspector decides whether a deficiency is a critical finding and needs immediate action using agency procedures. Usually this is easily determined, but occasionally the experience and judgment of a professional engineer may be required to reach a proper decision. A large hole through the deck of a bridge obviously needs attention, and a recommendation for immediate action is in order. Communicate the critical finding immediately and document actions taken in the report. By contrast, a slightly deteriorated bridge bearing may not be critical. A condition such as this would appropriately call for a recommendation for a preservation action.

Typically, most work recommendations submitted by the bridge inspector will be in the category of non-critical work. The recommended work is carefully described in the report along with a cost estimate.

The recommendations section of the report summarizes the following:

- Further inspection
- Maintenance
- Repairs
- Painting
- Posting
- Rehabilitation
- Replacement

Some state and local agencies designate separate personnel, not the inspector in the field, to prepare recommendations and cost estimates.

Report Appendices

To achieve maximum effectiveness of the inspection report, the report appendices contain any back-up information used to substantiate the inspector's findings, conclusions (if included) and recommendations. Typically, the appendices include photographs, drawings and sketches, and inspection forms. See Topic 4.4 for record keeping and documentation. Note that for routine inspections, inspection forms comprise the report, itself. Appendices may also include copies of any field notes used and specialist reports (e.g., underwater, nondestructive evaluation (NDE), and survey), or these documents may be referenced in the report. Although typically not conducted for routine inspections, a load capacity rating analysis of the structure may also be incorporated into the report appendices if performed. It is important to have the inspection report and all supplemental information, including report appendices, accurate with clear and concise descriptions or explanations.

Photographs

Photographs are a great asset to anyone reviewing reports on bridge structures. It is recommended that pictures be taken of any problem areas. Take pictures even if you think you can explain it completely in writing. It is better to take several photographs that may be considered unessential than to omit a photograph that could cause misinterpretation or misunderstanding of the report. At least two general photographs of every structure are provided in the appendix. One of these depicts the structure from the roadway, while the other photo is a view of the side

elevation (see Figures 4.6.1 and 4.6.2). Captions are provided for each photograph. Photographs are numbered so that they can be referred to in the body of the report. Sketches may also be a substitute for missing as-built plans.

Drawings and Sketches

Sketches and drawings needed to illustrate and clarify conditions of structural elements or serve as as-built plans are included or referenced. Sketches may be able to convey information not readily identified in a photograph (i.e., remaining web thickness). Original drawings are very helpful during future investigations with determining the progression of defects and to help determine any changes and their magnitude. Drafting-quality plans and sketches, sufficient to indicate the layout of the bridge and bridge site, may be included as an appendix.

Some reports combine photographs and sketches or text boxes together to accurately describe and document a particular deficiency.

Inspection Forms

The inspection forms comprise the actual routine inspection report and contain the field notes, as well as the numerical condition and appraisal ratings by the inspector. The inspection forms are normally signed by the inspection team leader. A complete SI&A form or equivalent is included in the appendix. Compare previous inspection forms to current conditions for inventory data accuracy.

Load Capacity Analysis

A load rating analysis may or may not be performed on the structure to determine the load-carrying capacity of the bridge. For routine inspections without a load capacity analysis, the results of the previous load capacity analysis are typically included in the report. If a load capacity analysis is performed, it is normally performed by engineers in the office, not by the inspector, and represents an investigation of primary load-carrying members of the bridge. A new load rating analysis is performed if the condition of the primary members has changed considerably since the last inspection. The report also includes recommendations for a new load rating analysis when maintenance or improvement work, change in strength of members, or dead load has altered the condition or capacity of the structure.

Field Inspection Notes

Include the original notes taken by the inspectors in the field or photocopies thereof in the appendix section of the report. The original field notes are source documents and as such are typically included in the bridge record.

Underwater Inspection Report

If an underwater inspection of the substructure has been performed, the summary of findings of the underwater inspection report (typically prepared by the dive team) is usually included in the appendix or cross-referenced to another location of the report.

4.6.4

Importance of the Inspection Report

Source of Information

A well-prepared report will not only provide information on existing bridge and bridge site conditions, but it also becomes an excellent reference source for future inspections, comparative analyses, and bridge study projects. Any conditions that are suspicious but unclear are reported in a factual manner, avoiding speculation. Terms such as “hazardous” or “dangerous” are subjective and are not used in the inspection report or inspection documentation that may be included in the appendix. Further action on such reports will be determined after review and consultation by experienced personnel.

Legal Document

In preparing an inspection report, keep in mind that bridge funding may be allocated or repairs designed based on this information. Furthermore, the inspection report is a legal record which may form an important element in future litigation. The language used in reports needs to be clear and concise and, in the interest of uniformity, care needs to be taken to avoid ambiguity of meaning. The information contained in reports is obtained from field investigations, supplemented by reference to “as-built” or “field-checked” plans. The source of all information contained in a report needs to be clearly stated.

Some state agencies require inspection reports to be signed, dated and sealed by a professional engineer before accepting them. Other state agencies require inspection reports to be signed and dated by the inspection team leader. The AASHTO *MBE* states (per Article 2.2) that "the components of data entered in a bridge record should be dated and include the signature of the individual responsible for the data presented." No undocumented alterations are allowed to the report once it is accepted. Some inspectors retain copies of their reports for their personal files in the interest of self-protection if there is any litigation.

Critical Findings

Critical findings are documented in the inspection report. However, the inspection report does not provide guidance for the follow-up to critical findings - the inspector does not wait for the inspection report to communicate and take action on critical findings. Instead, the follow-up to critical findings is a separate procedure that is immediately communicated with action taken on the critical findings, in accordance with the requirements of the NBIS. Agency procedures are established to assure that critical findings are addressed in a timely manner. In many instances when the critical finding exists, a plan of action is established and the deficiency is addressed prior to the formal submittal of the inspection report.

The FHWA is periodically notified of the actions taken to resolve or monitor critical findings. Advanced inspection methods for one or more elements may be recommended. The report provides information which may lead to decisions to limit the use of a bridge or close it to traffic; any bridge which the inspection has revealed to be a potential public safety concern.

Maintenance

Another purpose of the inspection report is to provide useful information about the needs and effectiveness of preservation activities. An active preservation program is vital to the long-term structural integrity of a bridge. The inspection report enables bridge preservation to be programmed more effectively through early

detection of structural deficiencies, therefore minimizing more costly future work and inconvenience to the traveling public.

Load Rating Analysis

When an inspection reveals deficiencies that may affect the load-carrying capacity of the structure, the findings need to be reviewed by an engineer to determine if a revised load rating analysis is required. A new load rating analysis is performed to determine the safe load capacity for the current condition. It may then be necessary to restrict loads crossing the bridge so that its safe load capacity is not exceeded. It is important that the revised load-carrying capacity (load rating) analysis become part of the bridge record.

Bridge Management

Another purpose of the inspection report is analysis by the bridge owners and the FHWA of the SI&A data. The intent of the analysis is to aid in the decisions for allocating and prioritizing funding.

Another important purpose of the inspection report is the data the report provides for use by the owner in managing the bridge asset. The data provided in the inspection report is important for the identification, prioritization, budgeting and programming of bridge preservation, improvement and replacement work. On a national level the data is used for reporting to Congress on the condition and performance of the Nation's bridges and for determining current and future estimates of funding needs. Furthermore, the data is used to: classify bridges according to serviceability, safety, and essentiality for public use; assign each a priority for replacement or rehabilitation; and determine the cost of replacing each such bridge with a comparable facility or of rehabilitating such bridge.

4.6.5

Quality

The accuracy and uniformity of information collected and recorded is vital for the management of an owner's bridges for preservation, improvement and replacement, and, most importantly, public safety. Quality cannot be taken for granted. The responsibility of ensuring quality bridge inspections rests with each bridge owner and the inspection team. Two phrases are frequently used when discussing quality; they are quality control and quality assurance.

NBIS regulations require each state to assure that systematic quality control (QC) and quality assurance (QA) procedures are being used to maintain a high degree of accuracy and consistency in the inspection program. Include periodic field review of inspection teams, periodic bridge inspection refresher training for program managers and team leaders, and independent review of inspection reports and computations.

The AASHTO *MBE* provides guidance for the implementation of appropriate quality control and quality assurance procedures. Quality control procedures include the "use of checklists to ensure uniformity and completeness, the review of reports and computations by a person other than the originating individual, and the periodic field review of inspection teams and their work." Quality assurance procedures include the "overall review of the inspection and rating program to ascertain that the results meet or exceed the standards established" by the bridge-owning agency.

Follow state-wide or agency-wide QC/QA procedures for a higher degree of accuracy and consistency in the inspection program.

See Topic 1.3 for a detailed description of quality control and quality assurance.

Appendix A

Sample Inspection Report

**PORT AUTHORITY OF ALLEGHENY COUNTY
PITTSBURGH, PENNSYLVANIA**

**REPORT ON THE
NBIS INSPECTION
OF
CHARTIERS CREEK BRIDGE**

BMS No. 02 7421 0000 9061

Submitted By:

Michael Baker Jr., Inc.
100 Airside Drive
Coraopolis, Pennsylvania
15108

September, 2011

STRUCTURE B.M.S. NUMBER: 02 7421 0000 9061

BRIDGE NAME: Chartiers Creek Bridge

LOCATION: Crafton, Pennsylvania

INSPECTION DATE: June 23, 2011

INSPECTED BY: Michael Baker Jr., Inc.
Patrick A. Leach, P.E.
Charles L. Molnar

PREPARED FOR: Port Authority of Allegheny County

PREPARED BY: Michael Baker Jr., Inc.
Written By: Joseph E. Salvadori, E.I.T.
Reviewed By: Raymond A. Hartle, P.E.

**PORT AUTHORITY
AGREEMENT NUMBER:** 11-08

OWNER OF BRIDGE: Port Authority of Allegheny County

COST INFORMATION:

Inspection & Report	\$4,662.00
Rigging	\$2,340.00
Traffic Control	\$ 0
Railroad	\$ 0
Insurance	\$ 0

DATE SUBMITTED: (Seal removed for BIRM)

TABLE OF CONTENTS

I Location Map

II Introduction

III Inspection Findings

- Inspection Summary
- Photographs
- Drawings (*Note – Drawings for this structure are not included in this example.*)
- Forms D-450's

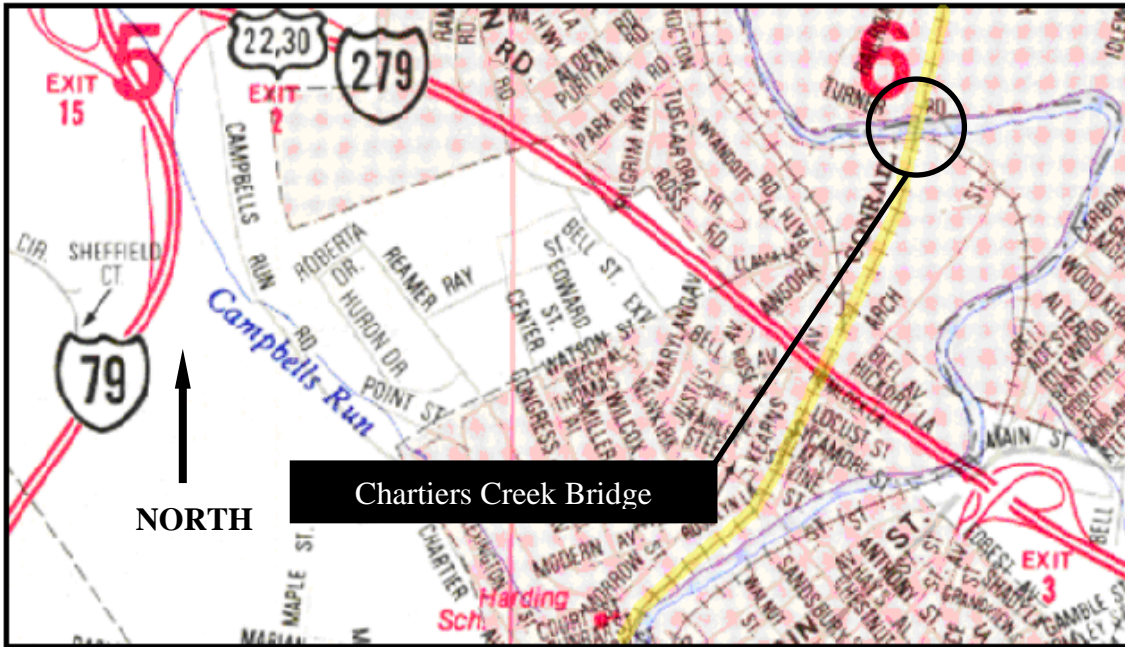
IV Structural Analysis

V Recommendations And Cost Estimate

VI Appendix

- BMS Forms D-491's (*Note – Not included in this example.*)

Chartiers Creek Bridge



Location Map
(No Scale)

**REPORT ON THE INITIAL NBIS INSPECTION
OF
CHARTIERS CREEK BRIDGE**

PORT AUTHORITY OF ALLEGHENY COUNTY

II. INTRODUCTION:

- Location

Located in the Borough of Crafton, the Chartiers Creek Bridge carries two (2) lanes of the Port Authority of Allegheny County's West Busway over Chartiers Creek, and the Pittsburgh Industrial Railroad, Inc.

- Year Built

The approximate date of the original construction of the Chartiers Creek Bridge is 1948. The structure was built by the Pennsylvania Railroad Company. Rehabilitation was completed in July 1997.

- Load Posting

None required.

- Description

The Chartiers Creek Bridge is a three (3) span, non-composite, riveted and bolted built-up plate girder bridge with a total length of 253' - 11" (see photo no. 1). The 3 spans consist of one (1) main simple span 124' - 0", one (1) simple south end span 55' - 3", and one (1) simple north end span 68' - 3" long. The span lengths are measured between centerline of bearings. The skew angle measured between the centerline of the abutment and West Busway is 90°. There are AT&T conduits mounted under the deck, and light poles mounted on top of the concrete parapets (see photo no.'s 8 & 2, respectively).

Chartiers Creek Bridge

The superstructure consists of four girders spaced at 7'-0" – 6'-0" – 7'-0" on centers, are laterally restrained with angle cross framing, and support an 8 1/2" reinforced concrete deck. The deck thickness includes a 1/2" integral-wearing surface. The deck measures 28'-0" between the reinforced concrete parapets present on both sides of the structure. Galvanized stay-in-place deck forms are present on the underside of the deck (see photo no. 8).

Span 1 girders are made up of a 5'-11" deep by 1/2" thick web plates, and 18" wide by 3/4" thick top and bottom flange plates (see photo no. 8). The main span consists of a 10'-4 1/2" deep by 1/2" thick web plate, and top and bottom flange plates varying from 20" wide by 7/8" thick, to 20" wide by 1" thick (see photo no. 9). Span 3 girders are made up of a 6'-10 1/2" deep by 1/2" thick web plate, and 18" wide by 3/4" and 7/8" thick top and bottom flange plates (see photo no. 10). New knee brackets, bolted to the fascia girders, measure 4'-9" wide, from the centerline of existing fascia girders to the centerline of the new W24x55 fascia stringers, with 1/2" thick web plates, and 6" wide by 1/2" thick top and bottom flange plates (see photo no. 4). Lateral bracing and diaphragms consist of angles, and angle x-bracing, respectively. Laminated elastomeric bearing pads are present at the girder ends.

The main span vertical underclearance, from the existing concrete channel bottom, at the centerline of the railroad measures 60'-9" and 36'-7" in span 1.

Gravity type substructures consist of a combination of original stone construction with newly constructed reinforced concrete abutment backwalls and pier caps (see photo no.'s 4 to 7).

Chartiers Creek Bridge

III. INSPECTION FINDINGS:

Michael Baker Jr., Inc. performed this initial inspection, which follows NBIS procedures, on June 23, 2011, via a UB-40 underbridge inspection crane. In general, the structure was in good condition with a few minor problems. Several conduits at the south abutment and in span 1 have severely buckled segments, and broken couplers and/or adapters (see photo no.'s 12 & 13). In addition, a conduit in span 3 is split and leaking water (see photo no. 14). These problems are due to the junction boxes being allowed to fill with rainwater during construction.

Approach

The north and south approach roadway and slabs are newly constructed with no deficiencies noted.

Deck

No deficiencies noted – new construction (see photo no. 11). All PennDOT Type 1 scuppers are in excellent condition. A few scuppers exhibit minor debris accumulation but are fully functional (see photo no. 15). Random hairline (< 0.01”) shrinkage cracks along the length of the concrete parapets are present (see photo no. 16). Deck expansion joints consist of strip seals in good condition with minor debris accumulation (see photo no. 17).

Superstructure

The superstructure has no visible structural deficiencies. Girders, fascia stringers, knee brackets, and lateral bracing are newly painted. The paint shows no visual defects, but the girders and bracing exhibit evidence of prior minor section loss and member pitting. Fascia stringers and knee brackets are in new condition with no deficiencies noted (see photo no. 4). Diaphragms are in good condition, but show areas of freckled surface rust under the broken

Chartiers Creek Bridge

conduit in span 1. Approximately 50% of lateral bracing connections between girders 3 & 4, in span 2, were not painted with final paint coat (see photo no. 18). Laminated elastomeric bearing pads are functioning properly with no problems noted.

Substructure

The north and south abutments are in good condition, with a few minor problems noted. Both abutments have newly constructed reinforced concrete backwalls, bridge seats, and wingwalls with no visual deficiencies noted (see photo no.'s 4 & 5). The stem tops consist of new reinforced concrete construction, also with no visual deficiencies noted, and are attached to the existing stone masonry bases. Some locations of the stone masonry show minor cracking and loosening of mortar.

Piers 1 & 2 are in good condition with minor cracking and loosening of mortar on the existing stone masonry portion of the stems. The bridge seats, caps, and stem tops are newly constructed reinforced concrete with no visual deficiencies noted (see photo no.'s 6 & 7).

Chartiers Creek Bridge



Photo No. 1 General Elevation (Upstream)



Photo No.2 South Approach (near)

Chartiers Creek Bridge



Photo No.3 North Approach (far)



Photo No.4 South Abutment (near) - Elevation

Chartiers Creek Bridge



Photo No.5 North Abutment (far) - Elevation



Photo No.6 Pier 1 - North Face (Looking South)

Chartiers Creek Bridge



Photo No.7 Pier 2 - North Face (Looking South), note electrical lines



Photo No.8 General Underside View - Span 1

Chartiers Creek Bridge



Photo No.9 General Underside View – Span 2



Photo No. 10 General Underside View – Span 3

Chartiers Creek Bridge



Photo No. 11 General Deck View



Photo No. 12 Conduit, Span 1 – note longitudinal crack/split

Chartiers Creek Bridge



Photo No. 13 Conduit and Couplers, Span 1 – note bend in conduit, and coupler separation

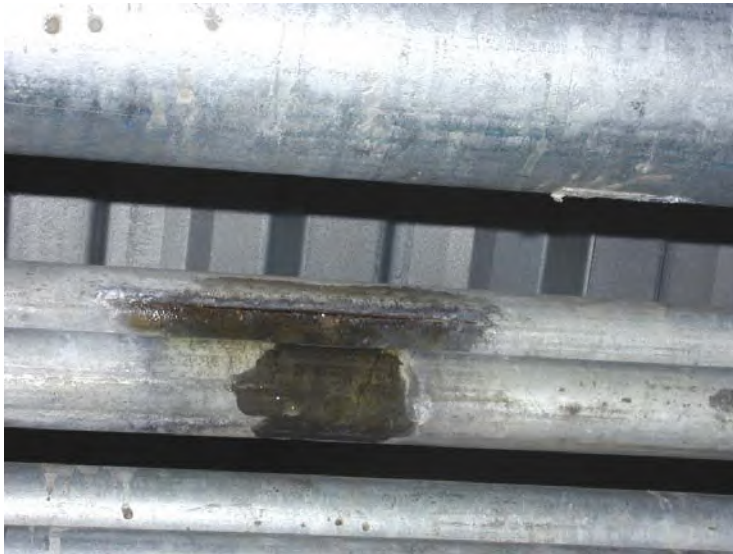


Photo No.14 Conduit , Span 3 – note conduit is split and leaking water

Chartiers Creek Bridge



Photo No. 15 Typical PennDOT Type 1 Scupper



Photo No.16 Typical parapet crack

Chartiers Creek Bridge

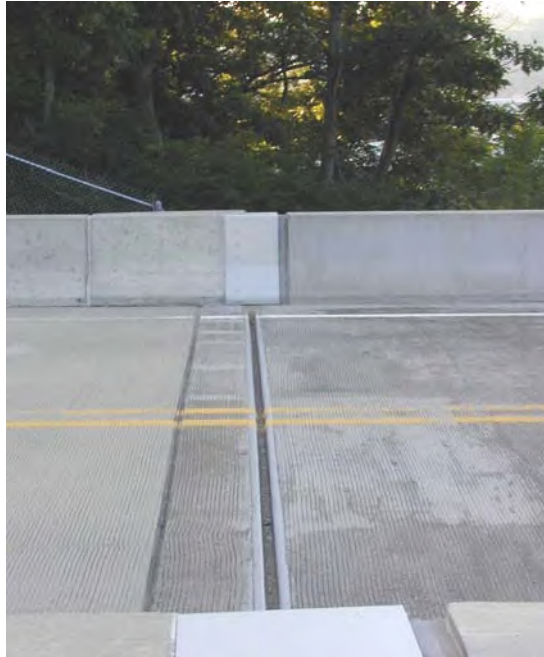


Photo No. 17 Strip Seal at North Abutment (typ.) – note minor debris accumulation



Photo No.18 Lateral bracing connection between beam #3 and #4, in span 2 – note no final paint coat, and rust freckles

Chartiers Creek Bridge

IV. STRUCTURAL ANALYSIS:

Bridge Load Ratings (Tons)

LOAD FACTOR	H	HS	ML	P
Inventory w/o F.W.S	115	159	152	---
Inventory w/ F.W.S	112	155	148	---
Operating w/o F.W.S	191	265	253	346
Operating w/ F.W.S	187	259	247	338

Note: 1) Critical rating is for a beam controlled by shear in span 3
 2) Due to no analysis being performed as part of the inspection, the above table is reproduced from contract drawings.

V. RECOMMENDATIONS AND COST ESTIMATE:

Repairs

Item	Estimated Quantity	Unit Cost	Total Cost
Drain junction boxes, and conduits filled with water. Repair bent conduits, and broken couplers/adapters.	N/A	Lump Sum	\$7,500.00
Paint locations requiring final paint coat between girders 3 & 4 in span 2.	20 SF	Lump Sum	\$1,500.00

TOTAL COST \$9,000.00

Note: The above costs are only for the items listed and do not include additional costs which would be incurred when the work is performed, such as mobilization, maintenance and traffic protection, engineering, etc.

Site Data

BRIDGE MANAGEMENT SYSTEM
BRIDGE INSPECTION REPORT

BMS Updated by _____ Date _____

A01 | **0** | **2** | **7** | **4** | **2** | **1** | **0** | **0** | **0** | **0** | **9** | **0** | **6** | **1** | **C05** Structure Type (Dept.)
Main **STL. RIVETED I-BEAM** | **1** | **9** | **1** | **1** | **0**

CHARTIERS CREEK BRIDGE Over **CHARTIERS CREEK** Approach _____

Inspection Date **E06** | **0** | **6** | **2** | **3** | **0** | **0** Name of Consultant and/or Inspectors **E12** | **M** | **I** | **C** | **H** | **A** | **E** | **L** | **B** | **A** | **K** | **E** | **R** | **J** | **R.** | **I** | **N** | **C.**

Inspection Type **E07** | **1** Inspected by **E08** | **8** Hired by **E13** | **8** Time started **7:30 A.M.** Weather Conditions: Temp: **84**

CRAFTON Time completed **4:30 P.M.** **MOSTLY SUNNY**

City Borough Township

Optional Reminder:
Check boxes if Maintenance Activities are needed -->

Bridge Signing Verification

BMS Item	Type of Sign	Required Sign	SIGNING IN FIELD			Comments
			Near Advance	Bridge Site Near Far	Far Advance	
D15	Bridge Weight Limit	N/A T				NONE POSTED
D15	Except Combination	N/A T				
D14	One Truck at a Time	Yes / (No)				
B22/B23	Vert. Clearance - On	N/A				See Sketch
B22/B23	Vert. Clearance - Und	N/A				See Sketch
	One Lane Bridge	Yes / (No)	(Opt)		(Opt)	
	Narrow Bridge	Yes / (No)	(Opt)		(Opt)	
	Hazard Clearance	Yes / (No)				
	Other					
(Opt)	Other					

Key --> OK: Signs properly installed M: Signs missing D: Signs damaged / incorrect New Wearing Surface Under Bridge: YES NO

Notes

Vert. Clear. Sign **On Feature:** **B01** = **B31** = **Under Feature:** **B01** = **B31** =

E26 Underclearance Appraisal **5** Controlling: Lateral **12'-2"** Vertical **36'-7"**

E28-A Traffic Safety Features (Subfields shown vertically) Posted Speed Limit _____ mph

6 Bridge Railing **PARAPET - JERSEY BARRIER. (GOOD CONDITION - MINOR CRACKING THROUGHOUT)**

8 Transition **PARAPET EXTENSIONS.**

8 Approach Guiderail **ON RIGHT - CONTINUOUS NJ BARRIER - GOOD. W-BEAM AND STL. POSTS ON NEAR LT. AND FAR LT.**

6 Approach Rail Ends **FLARED AND TURNED DOWN W-BEAM ON NEAR LT. AND FAR LT.**

E28 Approach Alignment **8** **NO SPEED REDUCTION. GOOD SIGHT DISTANCE.**

E15 Approach Roadway **8** **NEW PAVEMENT GOOD CONDITION.**

Pavement **GOOD**

Drainage **GOOD (ALL NEW CONSTRUCTION)**

Shoulders **GOOD**

E14 Approach Slab **8** **NEW CONSTRUCTION.**

Bump at Bridge Yes No

C19 Relief Joint **1**

Bridge 1 Data

Inspection Date

A01 0 2 7 4 2 1 0 0 0 0 9 0 6 1 E06 0 6 2 3 1 1

For Non-State Roadways

B01 Ref B27 ADT B28 ADT YR B30A ADTT %

For State highways, data from RMS will be used.

E25 Deck Geometry 6 Table Controlling Values: B27 / B34 / B22 A31 / A31 / B18 Design Exception granted?

E16 Deck Wearing Surface 9 NEW CONSTRUCTION (CONCRETE INTEGRAL)

C10 Wearing Surface Type 1 0 1 C10A Wearing Surface Thickness 0 5

E17 Deck 9 Estimated Spall or Delamination % Est. Chloride Content Top EXCELLENT CONDITION - NEW CONSTRUCTION.

Underside STAY IN PLACE FORMS (NO RUSTING NOTED) GALVANIZED AND IN GOOD CONDITION.

Exp Joint No. 4 C22 Exp Jt Types M B G GOOD CONDITION - SOME MINOR DIRT BUILD UP. (STRIP SEALS)

Deck Drainage GOOD - SOME SCUPPERS HAVE DEBRIS BUT NOT IN THE DOWNSPOUT.

E18 Superstructure 7 See Sheet for Additional Details. Form 491-J attached for FCM details Yes/No

Girders / Beams GOOD CONDITION - SUPERSTRUCTURE HAS BEEN RECONSTRUCTED FOR NEW BUSWAY BRIDGE. NEW PAINT/COATING OVER PREVIOUS PITTING/MORE SECTION LOSS. ALSO, SOME AREAS OVER LIGHT SURFACE RUST ON BOTTOM FLANGE. (THROUGHOUT)

Floorbeams N/A

Stringers NEW (FASCIA STRINGERS) W24 X 55 EXCELLENT CONDITION.

Diaphragms GOOD CONDITION. FEW AREAS OF FRECKLED SURFACE RUST UNDER BROKEN CONDUIT IN SPAN 1.

Truss Members N/A

Portals / Bracing FEW AREAS OF FRECKLED SURFACE RUST UNDER BROKEN CONDUIT IN SPAN 1. SEVERAL AREAS BETWEEN G3 AND G4 IN SPAN 2 WERE NOT PAINTED WITH FINAL COAT.

Bearings GOOD CONDITION. (LAMINATED ELASTOMERIC)

Drainage System (Below Deck) EXCELLENT CONDITION. (TYPE 1 SCUPPERS)

Abutment Data

(DEC 1996)

A01	0	2	7	4	2	1	0	0	0	0	9	0	6	1	E06	0	6	2	3	1	1
-----	---	---	---	---	---	---	---	---	---	---	---	---	---	---	-----	---	---	---	---	---	---

E20 Substructure **7** Details on Sheet

NAB - Near Abutment (Use same notation as W09)

Backwall GOOD CONDITION - NEW CONSTRUCTION.

Bridge Seats GOOD CONDITION - NEW CONSTRUCTION. VERY MINOR DEBRIS.

Cheekwalls

Stem GOOD CONDITION - NEW CONCRETE CONSTRUCTION AT TOP ON EXISTING STONE MASONRY BASE. SOME LOCATIONS HAVE MINOR CRACKING AND LOOSENING OF MORTAR.

Wings GOOD CONDITION - NEW CONSTRUCTION.

Footing NOT VISIBLE.

Piles NOT VISIBLE.

Scour / Undermine Yes No See Details on Form _____ Sheet _____

ABUTMENT IS NOT IN CHANNEL. ALSO, CHANNEL IS CONCRETE LINED.

Settlement NONE NOTED.

Embank-Slope-Wall GOOD CONDITION - HEAVY VEGETATION.

Wall Drainage

FAB - Far Abutment (Use same notation as W09)

Backwall GOOD CONDITION - NEW CONSTRUCTION.

Bridge Seats GOOD CONDITION - NEW CONSTRUCTION. MINOR DEBRIS.

Cheekwalls

Stem GOOD CONDITION - SAME AS NEAR ABUTMENT.

Wings GOOD CONDITION - NEW CONSTRUCTION.

Footing NOT VISIBLE.

Piles NOT VISIBLE.

Scour / Undermine Yes No See Details on Form _____ Sheet _____

ABUTMENT IS NOT IN THE CHANNEL.

Settlement NONE NOTED.

Embank-Slope-Wall HEAVY VEGETATION.

Wall Drainage

Pier Data

Inspection Date

A01	0	2	7	4	2	1	0	0	0	0	9	0	6	1	E06	0	6	2	3	1	1
-----	---	---	---	---	---	---	---	---	---	---	---	---	---	---	-----	---	---	---	---	---	---

Substructure (Cont.)

Pier / Bent Number 1 (Use same notation as W09)

Bridge Seats GOOD CONDITION - NEW CONSTRUCTION. 

Caps GOOD CONDITION - NEW CONSTRUCTION. 

Cheekwalls _____ 

Columns/Stems GOOD CONDITION - NEW CONSTRUCTION ON TOP OF EXISTING STONE MASONRY BASE. MINOR CRACKING AND LOOSE MORTAR. 

Footings NOT VISIBLE. 

Piles NOT VISIBLE. 

Scour / Undermine Yes No See Details on Form _____ Sheet _____ 

NOT IN CHANNEL - CHANNEL IS CONCRETE LINED.

Settlement NONE NOTED. 

Pier / Bent Number 2 (Use same notation as W09)

Bridge Seats GOOD CONDITION - NEW CONSTRUCTION. 


Caps GOOD CONDITION - NEW CONSTRUCTION. 

Cheekwalls _____ 

Columns/Stems GOOD CONDITION - SAME AS PIER 1. 

Footings NOT VISIBLE. 

Piles NOT VISIBLE. 

Scour / Undermine Yes No See Details on Form _____ Sheet _____ 

CHANNEL IS CONCRETE LINED.

Settlement NONE NOTED. 

Waterway 2 Data

U.W. Inspection Date

A01	0	2	7	4	2	1	0	0	0	0	9	0	6	1	W01-A					
-----	---	---	---	---	---	---	---	---	---	---	---	---	---	---	-------	--	--	--	--	--

W09 Substructure Unit	W10 Foundation Type	W11 Water Depth	W11-A Observed Scour Rating	W11-B U.W. Insp Performed	W11-C Observed Depth	W11-F Counter- Measures
P 0 2	P	0 0	9	E	0 0 0	

Findings: _____

W09 Substructure Unit	W10 Foundation Type	W11 Water Depth	W11-A Observed Scour Rating	W11-B U.W. Insp Performed	W11-C Observed Depth	W11-F Counter- Measures
F A B	P	0 0	9	E	0 0 0	

Findings: ABUTMENT OUT OF FLOOD PLANE.

W09 Substructure Unit	W10 Foundation Type	W11 Water Depth	W11-A Observed Scour Rating	W11-B U.W. Insp Performed	W11-C Observed Depth	W11-F Counter- Measures

Findings: _____

W09 Substructure Unit	W10 Foundation Type	W11 Water Depth	W11-A Observed Scour Rating	W11-B U.W. Insp Performed	W11-C Observed Depth	W11-F Counter- Measures

Findings: _____

W09 Substructure Unit	W10 Foundation Type	W11 Water Depth	W11-A Observed Scour Rating	W11-B U.W. Insp Performed	W11-C Observed Depth	W11-F Counter- Measures

Findings: _____

W09 Substructure Unit	W10 Foundation Type	W11 Water Depth	W11-A Observed Scour Rating	W11-B U.W. Insp Performed	W11-C Observed Depth	W11-F Counter- Measures

Findings: _____

Waterway 3 Data

(DEC 1996)

U.W. Inspection Date

A01	0	2	7	4	2	1	0	0	0	0	9	0	6	1	W01-A				
-----	---	---	---	---	---	---	---	---	---	---	---	---	---	---	-------	--	--	--	--

OBSERVED SCOUR RATING GUIDE

Rating	ITEM NUMBER								Rating
	1	2	3	4	5	6	7	8	
	Change Since Last Inspection	Scour Hole	Debris Potential	Substructure Scourability	Opening Adequacy/ Channel	Sediment	Alignment	Velocity/ Stream Slope	
9	None	None	None	NF/P9/R9	Good	None	Good	Low	9
8	None	Minor	None	P8/C8/R8	Good	Minor	Good	Low	8
7	Minor	Minor	Minor	P7/C7/R7	Fair	Minor	Good	Medium	7
6	Minor	Advanced	Medium*	A6	Fair	Medium	Medium	Medium	6
5	Medium*	Advanced	High*	A5	Fair	High	Medium	High	5
4	Medium	Serious*	High	R4*/A4*	Poor*	High	Poor*+	High	4
3	High*	Serious*	Present*	A3	Overtop*	High	Poor	High	3
2	Bridge is scour critical, IMMEDIATE action is required *								2
1	Bridge is scour critical, bridge is CLOSED *								1
0	Bridge has failed due to scour *								0

NOTES:

Rating considerations given in highest to lowest level of importance from left to right.
 * If an item is so marked, it cannot be given a higher ranking.
 s founded on competent rock and no problems exist.

C = Effective Countermeasures
 P = Pile Supported Substructures

DETERMINATION OF RATING FOR BMS ITEM

W11-A

Substructure Unit	1	2	3	4	5	6	7	8	W11-A
	Change Since Last Inspection	Scour Hole	Debris Potential	Scourability	Opening Adequacy/ Channel	Sediment	Alignment	Velocity/ Stream Slope	Overall Observed Scour Rating
P02	9	9	8	8	9	7	9	7	9

If Underwater Inspection only

Signatures and Date:

Bridge 2 Data

Inspection Date

A01 0 2 7 4 2 1 0 0 0 0 9 0 6 1 E06 0 6 2 3 1 1

E19 Paint Condition 8 8 New Paint Y/N If Yes: Spot Zone Full Revise item G08-G17

Interior Beam / Girder VERY GOOD - RECENTLY REPAINTED.

Fascias VERY GOOD - NEW.

Splash Zone: Truss / Girder _____

Truss _____

Bearings VERY GOOD.

Other _____

E23 Est. Remaining Life BMS to Calculate Yes/No 3 4 Comments _____

Recalculate IR/OR: Yes Due to: Deterioration New Wearing Surf. Other
No Previous Rating Dated _____ is still valid

E30 Inventory Rating 1 9 8 2 9 8 8 9 8 _____ _____ 2 9 8

E31 Operating Rating 1 9 8 2 9 8 8 9 8 _____ _____ 2 9 8
H HS ML-80 Other Other HS Load Factor

E32 Rate Meth 2 s E33 Typ Mem 1 AASHTO E37 Spec 9 4 E38 Manual 9 4

E29 Bridge Post 9 CONTROLLING: H _____ HS _____ ML80 Engineering Judgement _____

E24 Structural Condition Appraisal 7 Based upon Table 1 B27-ADT _____ B30-IR _____
or E18-Super 7 E20-Sub 7, E22-Culvert _____,

E01 Next Insp. Freq. 2 4 E03 Equip. Next Insp. B SNOOPER TRUCK (UB-40)

E04 Spec. Insp. Type E05 By Date _____

Is bridge over water? Yes. E22 = N Complete Forms D-450E through G
 No. E22 = N E21 = N E27 = N E29A = N

Notes: ONE SPAN IS OVER WATER AND ONE SPAN IS OVER RAILROAD.
HAD RAILROAD REPRESENTATIVE ON SITE. CREW WAS OUT OF SPAN 1 (RR LOCATION) BY TIME
REQUIRED. (9 A.M.)
INSPECTION WAS FIRST ON NEWLY CONSTRUCTED BUSWAY BRIDGE WHICH USED AN EXISTING RR BRIDGE.
CONDUITS ON BRIDGE WERE BUSTED AT ADAPTERS AT ABUTMENT 1. ALSO, ONE EXPANSION COUPLER WAS
BROKEN AND NEEDED REPLACED. SEVERAL CONDUIT SEGMENTS IN SPAN 1 WERE SEVERELY BUCKLED
AND NEEDED REPLACED.

Signatures and Date: PATRICK LEACH, P.E. - 6/23/11
CHARLES MOLNAR - 6/23/11

Maintenance Needs Data

Inspection Date

(DEC 1996)

A01
0
2
7
4
2
1
0
0
0
0
9
0
6
1
E06
0
6
2
3
1
1

H01
H03
H05
H08
H09
H01
H03
H05
H08
H09

Approach Roadway Work

Item #	Location	Quantity	PR	D/C
Pavement (Patch/Raise)	RDPAVMT L N R L F R	SY		
Pavement Relief Jt (Rep/Repl)	RDRLFJT L N R L F R	SY		
Shoulders (Repair/Reconstr)	RDSHLDR L N R L F R	SY		
Drainage-Off Bridge (Improve)	RDDRAIN L N R L F R	EA		
GR/Trans/End (Rep/Repl/Imp)	RDGDERL L N R L F R	EA		
Load Limit Signs (Replace)	RDLDSGN L N R L F R	EA		
Clearance Signs (Replace)	RDCLSGN L N R L F R	EA		
Cut Brush to Clear Signs	RDBRUSH L N R L F R	EA		
Approach Slab (Replace)	A744201 L N R L F R	SY		

Cleaning - Flushing

Item #	Location	Quantity	PR	D/C
Deck	A743101 -- --	EB		
Scupper/Down Spouting	B743101 1 2 3 4 5 0	EB		
Bearing/Bearing Seat	C743102 1 2 3 4 5 0	EB		
Steel-Horizontal Surfaces	D743102 1 2 3 4 5 0	EB		

Deck

Item #	Location	Quantity	PR	D/C
Bitum Deck W Surf (Rep/Repl)	BITWRGS 1 2 3 4 5 0	SY		
Timber Deck (Rep/Repl)	B744301 1 2 3 4 5 0	SY		
Open Steel Grid (Rep/Repl)	C744302 1 2 3 4 5 0	SY		
Concrete Deck (Repair)	D744303 1 2 3 4 5 0	SY		
Concrete Sidewalk (Repair)	E744303 1 2 3 4 5 0	SY		
Concrete Curb/Parapet (Rep)	F744303 1 2 3 4 5 0	SY		

Deck Joints - Expansion Joints

Item #	Location	Quantity	PR	D/C
Reseal	A743301 N 1 2 3 0 F	LF		
Repair/Reseal	A744101 N 1 2 3 0 F	LF		
Compression Seal (Rep/Rehab)	B744102 N 1 2 3 0 F	LF		
Modular Dam (Rep/Rehab)	C744102 N 1 2 3 0 F	LF		
Steel Dams (Rep/Rehab)	D744102 N 1 2 3 0 F	LF		
Other Types (Rep/Rehab)	E744102 N 1 2 3 0 F	LF		

Bridge Railings - Parapets

Item #	Location	Quantity	PR	D/C
Bridge Parapet (Rep/Repl)	RLGBRPR N 1 2 3 0 F	LF		
Struct Mount GR (Rep/Repl)	RLGSTRM N 1 2 3 0 F	LF		
Pedestrian (Rep/Repl)	RLGPEDN N 1 2 3 0 F	LF		
Median Barrier (Rep/Repl)	RLGMEDB 1 2 3 4 5 0	LF		

Deck Drainage

Item #	Location	Quantity	PR	D/C
Scupper Grate (Replace)	DRNGRAT 1 2 3 4 5 0	EA		
Drain/Scupper (Install)	B744401 1 2 3 4 5 0	EA		
Downspouting (Rep/Repl)	C744402 N 1 2 3 0 F	EA		

Bearings

Item #	Location	Quantity	PR	D/C
Lubricate	A743501 N 1 2 3 0 F	EA		
Steel (Rep/Rehab)	A744501 N 1 2 3 0 F	EA		
Steel (Replace)	B744501 N 1 2 3 0 F	EA		
Expansion (Reset)	C744502 N 1 2 3 0 F	EA		
Pedestal/Seat (Reconstruct)	D744503 N 1 2 3 0 F	EA		

Timber

Item #	Location	Quantity	PR	D/C
Stringer (Rep/Repl)	A744601 1 2 3 4 5 0	EA		
Other Members (Rep/Repl)	B744601 1 2 3 4 5 0	EA		

- REP..... Repair REPL..... Replace IMP..... Improve
- N..... Near UP..... Upstream LNR..... Near Left/Right
- F..... Far DN..... Downstream LFR..... Far Left/Right
- O..... Other UN..... Under 1,2,3, etc..... Span/Pier No.
- IN..... Inlet OUT..... Outlet EB..... Each Bridge (site)

MAJOR IMPROVEMENT NEEDS

F01 Year Needed
F04 Improvement Length

F02 Type Work
F06 Bridge Width

F10 Future ADT
F11 Future ADT Year

Reviewed On: _____

By: _____

Steel

Item #	Location	Quantity	PR	D/C
Stringer (Rep/Repl)	A744602 1 2 3 4 5 0	EA		
Floorbeam (Rep/Repl)	B744602 1 2 3 4 5 0	EA		
Girder (Repair)	C744602 1 2 3 4 5 0	EA		
Diaph/Lat. Bracing (Rep/Repl)	D744602 1 2 3 4 5 0	EA		

Reinforced, PS, PC, and PT Concrete

Item #	Location	Quantity	PR	D/C
Stringer (Rep/Repl)	A744603 1 2 3 4 5 0	EA		
Diaphragm (Rep/Repl)	B744603 1 2 3 4 5 0	EA		
Other Members (Rep/Repl)	C744603 1 2 3 4 5 0	EA		

Truss

Item #	Location	Quantity	PR	D/C
Members (Strengthen/Rep/Repl)	A744701 1 2 3 4 5 0	EA		
Portal (Modify)	B744701 1 2 3 4 5 0	EA		
Members (Tighten/Flameshorten)	C744702 1 2 3 4 5 0	EA		

Painting

Item #	Location	Quantity	PR	D/C
Superstructure - Spot	A743201 1 2 3 4 5 0	EB	1	4
Substructure - Spot	B743201 N 1 2 3 0 F	EB		
Superstructure - Full	C743201 1 2 3 4 5 0	EB		
Substructure - Full	D743201 N 1 2 3 0 F	EB		

Abutment - Wings - Piers

Item #	Location	Quantity	PR	D/C
Backwall (Rep/Repl)	A744801 L N R L F R	CY		
Abutments (Repair)	B744802 L N R L F R	CY		
Wing (Rep/Repl)	C744802 L N R L F R	CY		
Piers (Repair)	D744802 1 2 3 4 5 0	CY		
Footing (Underpin)	E744803 N 1 2 3 0 F	CY		
Masonry (Repoint)	F744804 N 1 2 3 0 F	LF		
Abut Slopewall (Rep/Repl)	A745101 L N R L F R	SY		
Abut Slopewall (Construct New)	B745102 L N R L F R	CY		
Pile Repair	A745901 N 1 2 3 0 F	EA		

Scour - Erosion Control

Item #	Location	Quantity	PR	D/C
Streambed Paving (Rep/Constr)	A745301 UP UN DN	CY		
Rock Protection	B745301 UP UN DN	CY		
Scour Hole (Backfill)	C745301 UP UN DN	CY		
Stream Deflector (Rep/Constr)	D745302 UP UN DN	CY		
Vegetation/Debris (Remove)	ECREMGV UP UN DN	CY		
Deposition (Remove)	ECREMDP UP UN DN	CY		

Culvert

Item #	Location	Quantity	PR	D/C
Headwall/Wings (Rep/Repl)	A745201 IN OUT	SY		
Apron/Cutoff Wall (Rep/Repl)	B745202 IN OUT	SY		
Barrel (Repair)	C745203 -- --	SY		

FOR COMPLETION BY REVIEW ENGINEER

Apply Protective Coating

Deck/Parapet/Sidewalk	A743401 DK PARA SW	SY		
Substructure	B743401 N 1 2 3 0 F	SY		

Construct Temporary

Support Pier	A745401 N 1 2 3 0 F	EA		
Pipe/Culvert Crossing	B745401 LT CL RT	EB		
Bridge	C745401 LT CL RT	EB		

PR - PRIORITY CODE

- 0 - Prompt action required. (Inform Bridge Engineer before updating BMS)
- 1 - High Priority, as soon as work can be scheduled.
- 2 - Priority, review work plan, adjust schedule if needed.
- 3 - Add to scheduled work.
- 4 - Routine structural, can be delayed until funds are available.
- 5 - Routine non-structural, can be delayed until programmed.

Chartiers Creek Bridge

Note: The Appendix section for this report is not included here. The BMS 491 Forms for PENNDOT are that state's version of the FHWA SI&A sheet with additional state items. The documents included in the report are typically red marked revisions to the file copy and reflect changes identified during the inspection.

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